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Exam = Final

QNO-1 :-

(a) Explain anti water logging measures-

Ans:- i) \Rightarrow Lining of canals and water courses-

it reduce seepage of water-

ii) \Rightarrow Reducing Intensity of irrigation:-

Only small portion of land should receive canal water in one particular

Season-

iii) \Rightarrow By introducing crop rotation:-

High water requiring crop should be followed

by one requiring less water and then

by requiring almost no water-

Example:- Rice followed by wheat and then by cotton-

iv) \Rightarrow Improving natural drainage of areas-
water should not be allowed to

②

To Stay in one area-

→ Neutral Flow is provided by bush and Jungle Cutting-

vi) ⇒ Economical use of water according to need-

vii) ⇒ Economical use of water according to need-

viii) Adoption of Sprinkler method of irrigation-

→ Only predetermined amount of water is supplied to land-

→ No percolation losses from water courses-

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QNO1-b) Different b/w Saline and alkaline soils:

⇒ SALINE: ① Saline soils are soil that have been harmed by soluble salts consisting mainly of sodium, calcium, magnesium, chloride and sulfate, bicarbonate.

2) They have less pH.

3) It is basic in nature.

4) Its pH is 7-8.5

5) Dominating compound is sodium salts.

⇒ ALKALINE SOIL: ① Alkaline soil are clay soil.

② They have high pH.

③ They are more basic.

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4) pH greater than 8.5

5) Dominating Compound in Sodium Compound in Sodium Carbonate.

QND:- c) How do you reclaim salt affected lands?

ANS:- ALKALI salts (Sodium chloride, Sodium Sulphate and Sodium Carbonate) are injurious to agriculture.

NaCl — Least harmful.

Na_2SO_4 — Medium harmful.

Na_2CO_3 — Most harmful.

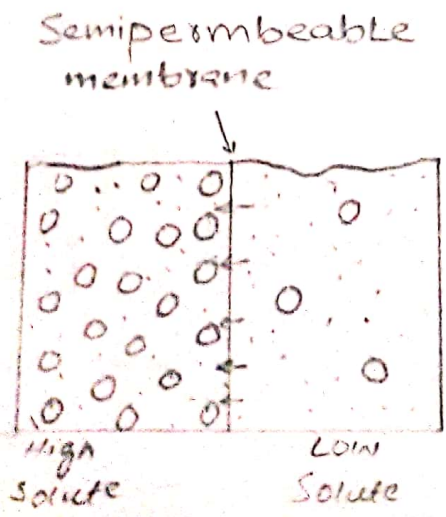
→ The above salts are soluble in water.

→ When W.T rises up or roots are in Capillary zone, the G.W moves upwards and salts are deposited in

in root zone and surface of soil.

The phenomena of salts coming up in solution and forming a thin crust (5-7.5cm) on the surface after evaporation of water is called Efflorescence.

Land affected by efflorescence is called saline soil. Salts surrounding the roots reduce the osmotic activity of plants.



⑥

QNO2 → a1 Explain the procedure of designing of an irrigation Canal by Kennedy's theory-

ANS: KENNEDY'S THEORY:

R. G. Kennedy studied straight reaches of upper Barf Daab Canal which are stable for 30 years-

$$\rightarrow V_0 = C D^n$$

where V_0 is critical velocity (non-silting or non-scouring)

C is constant depends upon quantity of silt-

Sediment is kept in suspension solely by the vertical component of eddies weight of sediment vertically acts downwards-

(B)

→ weight of sediments vertically acts downwards

→ vertical component of eddies acts upwards.

→ Result:- The sediment is in suspension

• $v_0 = 0.84 D^{0.64}$ FPS System.

• $v_0 = 0.546 D^{0.64}$ MKS System, D is depth.

• $v_0 = 0.546 D^{0.64}$ where $m = v/v_0 = \text{Critical}$

velocity ratio (C.V.R) depend upon silt grade.

• $v = \text{Critical velocity for all sizes of sediment, } v_0 \text{ is var for upper Bari}$

Doab Canal Only.

Kennedy procedure for Canal design.

Step 1:-

Assume the trial value of D and

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and put in eq 1 and determine

$$V_0 = 0.546mD^{0.64}$$

Step # 02:- in equation 1 $Q = AV$

$$A = \frac{Q}{V}$$

$$A = BD + D^{\frac{3}{2}}$$

$$P = B + D^{\frac{5}{2}}$$

For assumed D
determine B
find $R = A/P$

Step # 03:- Substitute the value of R in

equ. 2 (Kutter and Chazys Eqn) to

Obtain V which will be actual

velocity for assumed dimension.

Step # 04:- If the velocity worked out from

Eqn-2 agrees with that of obtained
with the Eqn-3 (Kennedy's Eqn) then
the assumed depth is correct other
wise repeat the procedure with
changed value of D

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QNO 2-(b) :- Given data:-

$$\text{Discharge (Q)} = 30 \text{ m}^3/\text{sec}$$

$$C_v (m) = 1 \quad N = 0.0225$$

Bed Slope = 1 in 5000

$$\text{Depth (D)} = 2.3 \text{ m}$$

Solutions:- finding velocity.

$$\text{By formula } V_k = 0.56 m D^{0.64}$$

$$= 0.56 (1) (2.3)^{0.64} \Rightarrow \underline{V_k = 0.930 \text{ m}}$$

→ NOW Calculating Area of Canal.

By formula.

$$Q = AV \Rightarrow A = Q/V = \frac{30}{0.930}$$

$$\underline{A = 32.25 \text{ m}^2}$$

→ NOW we have to Calculate B by

using formula $A = BD + \frac{D^2}{2} \rightarrow BD + 0.5D^2$

→ By putting values
 $A = BD + 0.5D^2$

$$32.25 = B(2.3) + 0.5(2.3)^2$$

$$B = \frac{29.605}{2.3} \Rightarrow \underline{12.87 \text{ m} = B}$$

→ NOW we have to Calculate wetted perimeter So By formula-

$$P = B + \sqrt{5} D$$

$$= 12.87 + \sqrt{5} (2.3) \Rightarrow \boxed{P = 18.01 \text{ m}}$$

→ Now we have to Calculate ~~the~~ Hydraulic Radius

$$R = \frac{A}{P} = \frac{32.25}{18.01}$$

$$\boxed{R = 1.79 \text{ m}}$$

→ NOW Calculating mean velocity from Chezy equation

$$V_L = C (RS)^{\frac{1}{2}}$$

Where $C = \frac{1}{\frac{1}{0.0225} + \left(23 + \frac{0.00155}{S}\right) \frac{n}{\sqrt{R}}}$

$$\Rightarrow \frac{1}{0.0225 + \left(23 + \frac{0.00155}{(1/5000)}\right)}$$

$$1 + \left(23 + \frac{0.00155}{(1/5000)}\right) \times \left(\frac{0.0225}{\sqrt{1.79}}\right)$$

$$C = \frac{75.19}{1.517} = 49.56 \Rightarrow V_L = 49.56 (1.79)^{\frac{1}{2}}$$

$$\boxed{V_L = 0.93 \text{ m}}$$

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QNO3 → 1a1: Differentiate between initial regime and final regime - in accordance to Lacey's theory.

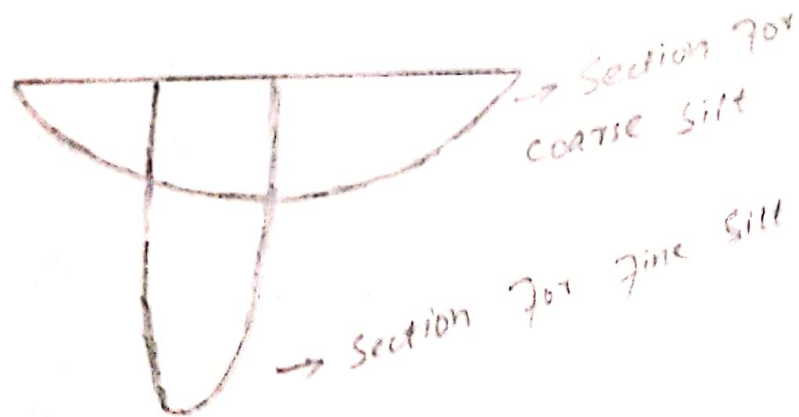
ANS: LACEY'S THEORY:-

- According to Kennedy, a channel is regime (NO silting NO scouring) but according to Lacey even through channel with no silting or scouring may actually be not in regime.

- He differentiated b/w initial regime and final regime but this theory is applicable to final regime.

→ INITIAL REGIME:- When only bed slope of channel but the cross section remains same then also no silting or scouring take place. But this is rare.

⇒ FINAL REGIME: if all the parameters (perimeter, depth and slope) have equally free to vary and adjust according to discharge and silt grades then the channel is said to have final regime.



03-(b) :- Given Data:-

Q = 30 m³/sec M = 0.56

Solutions:- $f = 1.76 M^{0.5} \Rightarrow 1.76 \times (0.56)^{0.5}$

F = 1.3

$v_m = \left[\frac{Q F^2}{140} \right]^{1/6} \Rightarrow \left(\frac{30 \times (1.3)^2}{140} \right)^{1/6}$

v_m = 0.844

Q = AV $\Rightarrow A = \frac{Q}{v} = \frac{30}{0.844}$

A = 35.55

$P = 4.75 \sqrt{Q} \Rightarrow P = 26.00$

$R = \frac{5}{2} \times \frac{v^2}{f} = \frac{5}{2} \times \frac{(0.844)^2}{1.3}$

R = 1.36

$A = BD + \frac{D^2}{2}$

$35.5 = BD + \frac{D^2}{2} \rightarrow (i)$

$P = B + D\sqrt{5}$

$26.01 = B + 2.236D \rightarrow (ii)$

put (i) and (ii)

$$35.54 = (26.01 - 2.236)D + \frac{D^2}{2}$$

$$35.54 = 26.01D - 1.736D^2$$

$$-1.736D^2 + \frac{26.01}{1}D - \frac{35.54}{1} = 0$$

$$a = -1.736 \quad b = 26.01 \quad c = -35.54$$

By Quadratic Eq

$$D = \frac{-(-26.01) \pm \sqrt{(26.01)^2 - 4(-1.736)(-35.54)}}{2(-1.736)}$$

$$|D = 1.52| \text{ put in eq (i)}$$

$$B = 26.01 - 2.236(1.52)$$

$$B = 22.611$$

$$S = \frac{f^{5/3}}{3.340 Q^{1/6}} = \frac{(1.3)^{5/3}}{3.340 (30)^{1/6}}$$

$$|S = 0.00026|$$

(15)

Q No. 121:- Explain the Components of headworks
with neat diagram.

Ans:- Components of a diversion headworks-

- weir or barrage.
 - Under sluices.
 - Divide wall.
 - Fish Ladder.
 - Canal head regulator.
 - River Training works (Marginal bunds and guide banks)
 - Silt excluders/silt precession devices
- ⇒ **weir**: Normally the water level of any perennial river is such that it cannot be diverted to the irrigation canal the bed level of the canal may be higher than the existing water level of the river in such cases weir is constructed across the river to raise the water level. Superior water pass over the crest of weir.

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→ **Barrage**:- When water level on the up stream side of the weir is required to be raised to different levels at different time barrage is constructed

Barrage is an arrangement of adjustable gates or shutters at different times over the weir-

→ **Under Sluices**:-

Also known as scouring sluices the under sluices are the openings provided at the base of the weir or barrage these openings are provided with adjustable gates

Normally the gates are kept closed the suspended silt goes on depositing in front of the canal head regulator-

→ **Divide wall:** The divide wall is a long wall constructed at right angles in the weir or barrage it may be constructed with stone masonry or cement concrete on the upstream.

→ **Fish ladder:** The fish ladder is provided just by the side of the divide wall for the free movement of fishes. River are important source of fishes.

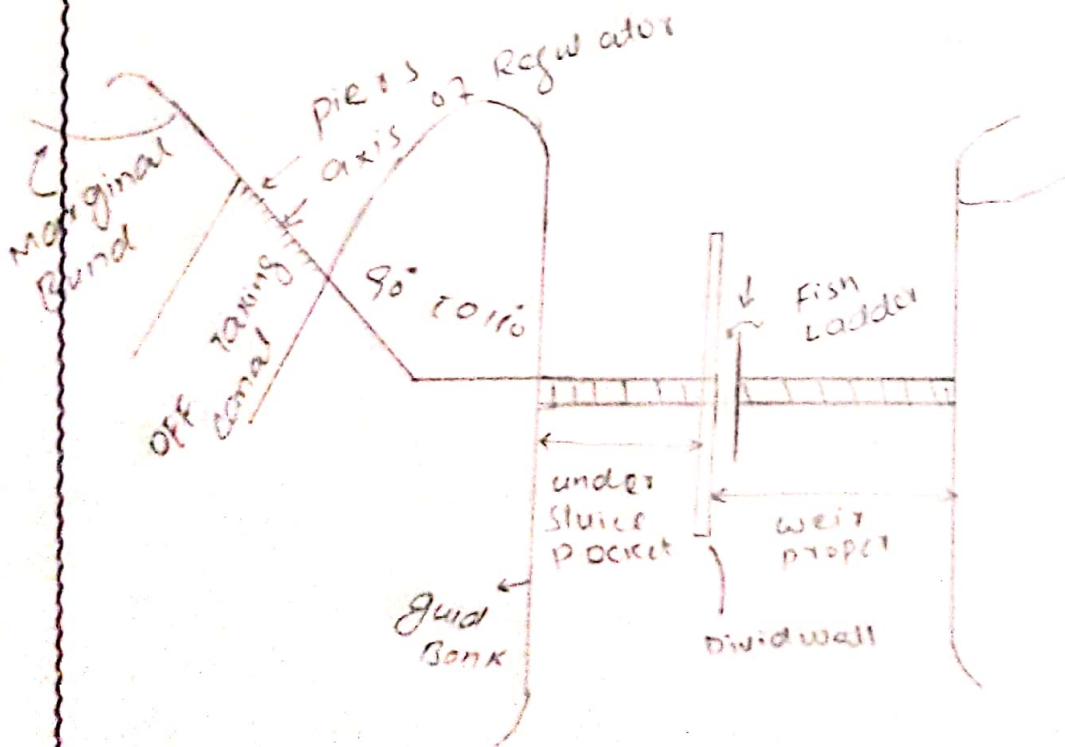
→ **Canal head Regulator:**

A structure which is constructed at the head of the canal to regulate flow of water is

known as canal head regulator. It consists of a number of piers which divide the total width of the canal.

⇒ Functions of Canal Head

Regulator: it regulates the supply of water entering the canal. It controls the entry of silt in the canal. It prevents the river floods from entering the canal.



Q No 4 (b) What are the Functions of Head regulators?

ANS:- Head regulator is constructed at the off taking are called head regulators-

→ when it is constructed at load of main canal it is known as canal head regulator

Function:-

To control the entry of water either from the reservoir or from the main canal

→ To control the entry of silt into off taking of main canal.

→ To serve as a meter for measuring discharge of water.