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Mid term Examination



Q No 1(a)

Define the following term

PLASTIC EQUILIBRIUM:

A body of soil is said to be in state of plastic equilibrium.

if every part of soil is on the verge of failure is called plastic equilibrium

Angular Distortion:

It is defined as

the ratio between the relative deflection between two points in a foundation and the distance between them is called angular distortion.

Compressive Index:

Soil compressive Index is define as

$$C_c = \frac{\Delta e}{\log_{10}(P_2/P_1)}$$

Δe = change in void ratio

P_1 = Pressure when void ratio is e_1

P_2 = when void ratio is e_2

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Ultimate Bearing Capacity:

It is defined as,

The gross pressure intensity at the base of foundation which would cause shear failure is called ultimate bearing capacity.

d) Poisson Ratio of Soil:

It is defined as,

Poisson ratio (μ) is the negative of ratio of transversal strain to the axial strain in an elastic material which is subjected to an uniaxial stress

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Q No 01(b)

A 6m tall cantilever wall retaining the soil that has the following properties

- $C = 0$
- $\phi = 30^\circ$
- $\gamma = 19.2 \text{ kN/m}^3$

And the ground surface behind the wall is inclined at a slope of 3 horizontal and 1 vertical. The wall has moved sufficiently to develop active condition. Determine the total normal and shear forces acting on the back of this wall using Rankine theory

GIVEN:

$$H = 6\text{m}, C = 0$$

$$d = 30^\circ, \gamma = 19.2 \text{ kN/m}^3$$

$$\text{Slope } H = 3, V = 1$$

Required:

$$\frac{N_a}{b} = ? , \frac{V_a}{b} = ?$$

SOLUTION:

$$\frac{P_a}{b} = \frac{\gamma H^2 + k_a}{2}$$

$$B =$$

$$\tan \beta = \frac{1}{3}$$

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$$B = \tan^{-1}(1/3)$$

$$\beta = 18^\circ$$

$$K_a = \cos \beta \times \frac{\cos \beta - \sqrt{\cos^2 \beta - \cos^2 \phi}}{\cos \beta + \sqrt{\cos^2 \beta - \cos^2 \phi}}$$

$$= \cos(18) \times \frac{\cos(18) - \sqrt{\cos^2(18) - \cos^2(30)}}{\cos(18) + \sqrt{\cos^2(18) - \cos^2(30)}}$$

$$K_a = 0.3948$$

$$K_a = 0.395$$

$$\frac{P_a}{b} = \frac{19.2 \times (6)^2 \times 0.395}{2}$$

$$\frac{P_a}{b} = 136.512 \text{ kN/m}$$

$$\frac{N_a}{b} = \frac{P_a}{b} \cos \beta$$

$$= 136.512 \times \cos(18)$$

$$= 129.83 \text{ kN/m}$$

$$V_{a/b} = \frac{Pa}{b} \sin \beta$$

$$= 136.512 \times \sin(18)$$

$$= 42.18 \text{ KN/m}$$

RESULT:

$$N_{a/b} = 129.83 \text{ KN/m}$$

$$V_{a/b} = 42.18 \text{ KN/m}$$

Q No 2(a)

What is bearing capacity? Also write factor affecting bearing capacity.

ANSWER:

BEARING CAPACITY:

the engineering property of the soil due to which it resist the applied load.

Denoted by q .

In other words the internal strength of soil is called bearing capacity.

Factor affecting bearing capacity:

- 1- Relative Density of soil.
- 2- Depth of footing
- 3- Breadth of footing
- 4- Unit weight of soil
- 5- Water Table.

RELATIVE DENSITY OF SOIL:

More relative density

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of soil more will be its angle of friction. More will be N_r, N_c, N_r , with increase of this bearing capacity increases.

DEPTH OF FOOTING:

With the increase of depth of foundation the bearing capacity of soil will increase.

This increase will be more if soil is dense

BREADTH OF FOUNDATION:

More the breadth of foundation more will be bearing capacity of soil.

UNIT WEIGHT OF SOIL:

Bearing Capacity of soil is directly proportional to unit weight of soil. The bearing capacity of soil increase with increase in unit-weight.

Water Table:

As water table come near surface bearing capacity decreases.

Q No 2(b)

What is maximum safe load which can be supported by rectangular footing 2m-by-3m with factor of safety 3. The base of the footing is at 1.6m below the ground surface.

The unit weight of soil is 18 kN/m^3 . The angle of shear resisting $\phi = 20^\circ$ ($N_c = 14.8$ $N_q = 6.4$ $N_\gamma = 2.9$) unit cohesion $c_u = 20 \text{ kN/m}^2$. Use Meyerhof analysis.

GIVEN DATA:

$$L = 3 \text{ m}, B = 2 \text{ m}$$

$$D = 1.6 \text{ m}$$

$$F.O.S = 3$$

$$\gamma = 18 \text{ kN/m}^3$$

$$\phi = 20^\circ$$

$$c_u = 20 \text{ kN/m}^2$$

REQ:

$$Q_s = ?$$

SOLUTION:

$$Q_u = c N_c s_c d_c + \sqrt{N_q} d_q s_q + \frac{1}{2} \gamma \cdot B \cdot N_\gamma \cdot d_\gamma \cdot s_\gamma$$

First for shape factor:

$$\alpha = \left(45 + \frac{\phi}{2} \right) = \left(45 + \frac{20}{2} \right)$$

$$\alpha = 55^\circ$$

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$$S_c = 1 + 0.2 (B/L) \tan^2 \alpha$$

$$1.02 (2/3) \tan^2(35)$$

$$S_c = 1.27 = 1.3$$

$$S_{qv} = S_s = 1 + 0.1 B/L \tan^2 \alpha$$

$$= 1.1 (2/3) \tan^2(35)$$

$$S_{qv} = S_s = 1.14$$

Depth factor:

$$d_c = 1 + 0.2 (D/B) \tan \alpha$$

$$= 1 + 0.2 (1.6/2) \tan(35)$$

$$d_c = 1.23$$

$$d_r = d_v = 1 + 0.1 (D/B) \tan \alpha$$

$$1 + 0.1 (1.6/2) \tan(35)$$

$$= 1.11$$

$$Q_u = C \cdot N_c \cdot S_c \cdot d_c \times q \cdot N_q \cdot d_q \cdot S_v + \frac{1}{2} \gamma \cdot B \cdot N_r \cdot d_r \cdot S_r$$

$$(20 \times 14.8 \times 1.3 \times 1.23) + (18 \times 1.6) \times 6.4 \times 1.11 \times 1.14$$

$$+ (0.5 \times 20 \times 2 \times 2.9 \times 1.11 \times 1.14)$$

$$Q_u = 762 \text{ KN/m}^2$$

$$Q_{n.u} = Q_u - \bar{\delta}$$

$$= 762 - (18 \times 1.6)$$

$$\bar{\delta} = 8 \times D$$

$$= 733.2 \text{ KN/m}^2$$

$$Q_{n.s} = \frac{Q_{n.u}}{F.O.S}$$

$$= \frac{733.2}{3}$$

$$= 244.4 \text{ KN/m}^2$$

$$Q_s = Q_{n.s} + \delta$$

$$244.4 + (1.6 \times 18)$$

$$Q_s = 273.2 \text{ KN/m}^2$$

Total safe load on Rectangular footing

$$A \times Q_s = (2 \times 3) \times 273.2$$

$$= 1639.2 \text{ KN}$$

Q No 3 (a)

What is settlement? Explain types in detail

ANSWER:

SETTLEMENT:

When load is applied on the ground surface this will produce effective vertical ~~stresses~~ stresses, due to these stresses effective vertical strain will be produced as a result of which the movement will occur in downward direction. This downward movement is called settlement.

Types of Settlement:

On the basis of movement of the structure it is divided into two types

- i) Total Settlement
- ii) Differential Settlement

Total Settlement:

It is also called uniform settlement. In this type of settlement each part of structure will settle equally.

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In uniform settlement the failure of the structure is not much as considered as with differential settlement.

The total settlement mostly take place in the structure which are constructed on rigid footing

Differential Settlement:

Different settlement in different part of same structure is called differential settlement

Differential settlement is more danger than total settlement.

It cause more damage to structure.

Types of differential settlement:

two types

1- Tilt

2- Angular distortion

Q No 3(b)

A soil has compressive index $C_c = 0.31$. At a stress 130 kN/m^2 , the void ratio was 1.02. Calculate

- The void ratio if the stress on the soil is increased to 170 kN/m^2
- The total settlement of the stratum of 5m thickness.

GIVEN:

$$C_c = 0.31$$

$$P_1 = 130 \text{ kN/m}^2$$

$$e_2 = 1.02$$

$$P_2 = 170 \text{ kN/m}^2$$

$$H = 5 \text{ m}$$

Required:

Void ratio, $e_1 = ?$

$S_c = ?$

SOLUTION:

$$C_c = \frac{\Delta e}{\log_{10}(P_2/P_1)}$$

$$C_c = \frac{e_0 - e_1}{\log_{10}(P_2/P_1)}$$

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$$0.31 = \frac{1.02 - e_1}{\log_{10} \left(\frac{170}{130} \right)}$$

$$e_1 = 0.54$$

Now

$$S_c = \frac{H}{1 + e_0} \times C_c \log_{10} \left(\frac{P_2}{P_1} \right)$$

$$= \frac{5}{1 + 1.02} \times 0.31 \log_{10} \left(\frac{170}{130} \right) \times 1000 \text{ mm}$$

$$S_c = 1003.427$$

RESULT:

$$e_1 = 0.54$$

$$S_c = 1003.427$$