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MID Term Exam

Q1) a) Define the following terms:-

1) Plastic Equilibrium:-

It is defined as "The state of stress within a soil mass that has been deformed to such an extent that its ultimate shearing resistance is mobilized."

2) "Ultimate Bearing Capacity:-"

It is defined as "The maximum gross pressure intensity at the base of foundation at which the soil

does not fail in shear when the term bearing capacity is used.

(2)

3) "Compressive Index:-"

Compressive Index is used to find the settlement in the normally consolidated clay. The total stress applied is larger than the stress in field due to which the soil sample has been undergone. This type of clay soil is said to be normally consolidated soil.

4) "Poisson Ratio of Soil:-"

It is defined as

"The negative ratio of transverse stress to the axial strain in the elastic material which is subjected to uniaxial stress."

5) "Angular Distortion:-"

When two foundation supports wall and column settle unequally, it means the structure is subjected to angular distortion.

Q1b) "Given Data:-"

$$H = 6m$$

$$C = 0$$

$$\phi = 30^\circ$$

$$\text{slope} = \text{Horizontal} = 1$$

$$\text{Verticle} = 3$$

"Required:-"

$$\frac{P_a}{b} = ?$$

$$\frac{V_a}{b} = ?$$

Sol:-

$$\frac{P_a}{b} = \frac{\gamma \times H^2 \times K_a}{2}$$

$$B = \tan^{-1}\left(\frac{1}{3}\right)$$

$$B = 18^\circ$$

$$K_a = \cos B \times \frac{\cos B - \sqrt{\cos^2 B - \cos^2 \phi}}{\cos B + \sqrt{\cos^2 B - \cos^2 \phi}}$$

$$K_a = \cos(18) \times \frac{\cos(18) - \sqrt{\cos^2(18) - \cos^2(30)}}{\cos(18) + \sqrt{\cos^2(18) - \cos^2(30)}}$$

$$K_a = 0.394$$

$$K_a = 0.395$$

$$\text{Now } \frac{P_a}{b} = \frac{19.2 \times (6)^2 \times 0.395}{2}$$

$$= 136.512 \text{ kN/m}$$

(4)

$$\frac{N_a}{b} = \frac{P_a}{b} \cos \beta$$
$$= 136.512 \times \cos(18)$$

$$\frac{N_a}{b} = 129.8 \text{ kN/m}$$

$$\frac{V_a}{b} = \frac{P_a}{b} \sin \beta$$
$$= 136.512 \sin(18)$$

$$\frac{V_a}{b} = 42.18 \text{ kN/m}$$

Q2a) What is bearing Capacity, Also write factors effecting bearing Capacity.

Bearing Capacity:-

It is an engineering property of soil due to which the soil resist the applied load.

- The bearing Capacity is also known as the internal strength.
- It is denoted by "q"

"Factor Affecting Bearing Capacity:-"

Following are the factor which affects the bearing Capacity-

1) ⁵Relative Density:-

- Greater the relative density of soil higher will be the value of angle of internal friction ϕ . Higher the value of internal friction higher will be the value of Terzaghi Bearing factor.
- Greater the value of N_c, N_r, N_q will result in high value of bearing capacity.
- The soil have enough relative Density will have enough bearing capacity.
- Relative Density $\Rightarrow \gamma_{\text{relative}} = \frac{e_m - e}{e_{\text{max}} - e_{\text{min}}}$

2) ⁴Depth of Footing:-

- The bearing capacity of soil increases with the increase of depth of footing.
- This increase will be maximum for the dense soil as compared to loose soil.

3) ³Unit Weight of Soil:-

Bearing Capacity of soil is directly proportional to unit weight of soil. The bearing capacity of soil increase with increase in its unit weight.

4) ²Water Table:-

As the water table comes near to footing, the bearing capacity get decreases.

5) "Breadth of footing:-" (b)

foundation of soil. \Rightarrow More the breadth of footing or more will be the bearing capacity

\Rightarrow It will be more in case of dense soil/sand as compared with loose or medium soil.



Q2(b):- "Given Data:-"

$$L = 3\text{m}$$

$$B = 2\text{m}$$

$$D_f = 1.6\text{m}$$

$$F.O.S = 3$$

$$\gamma = 18\text{KN/m}^3$$

$$c = 20\text{KN/m}^2$$

$$\phi = 20^\circ$$

"Required:-"

$$q_u = ?$$

"Solution:-"

$$q_u = C N_c \cdot S_c d_c i_c + \gamma N_q \cdot S_q d_q i_q + \frac{1}{2} \gamma N_r \cdot S_r d_r i_r$$

"First for the shape factor:-"

$$\alpha = \left(45 + \frac{\phi}{2} \right)$$

$$= \left(45 + \frac{20}{2} \right) = 55^\circ$$

$$\begin{aligned}
 S_c &= 1 + 0.2 \frac{B}{L} \tan^2 \alpha \\
 &= 1 + 0.2 \left(\frac{2}{3}\right) \tan^2 55 \\
 &= 1.27 \approx 1.3
 \end{aligned}$$

$$\begin{aligned}
 S_q = S_r &= 1 + 0.1 \frac{B}{L} \tan^2 \alpha \\
 &= 1 + 0.1 \frac{2}{3} \tan^2 55 \\
 &= 1.14
 \end{aligned}$$

"Depth Factor:"

$$\begin{aligned}
 d_c &= 1 + 0.2 \frac{D}{B} \tan \alpha \\
 &= 1 + 0.2 \frac{1.6}{2} \tan 55 \\
 &= 1.23
 \end{aligned}$$

$$\begin{aligned}
 d_r = d_q &= 1 + 0.1 \frac{D}{B} \tan \alpha \\
 &= 1 + 0.1 \left(\frac{1.6}{2}\right) \tan 55 \\
 &= 1.11
 \end{aligned}$$

Now putting values.

$$\begin{aligned}
 q_u &= (20 \times 14.8 \times 1.3 \times 1.23 \times 1) + (18 \times 1.6 \times 6.4 \times 1.11 \times 1.11 \times 1) + (0.5 \times 20 \times 2 \times 2.9 \times 1.11 \times 1.14 \times 1) \\
 q_u &= 762 \text{ kN/m}^2
 \end{aligned}$$

(8)

$$\begin{aligned}q_{n.u} &= q_u - \bar{s} \\ &= 762 - (18 \times 1.6) \\ &= 733.2 \text{ kN/m}^2\end{aligned}$$

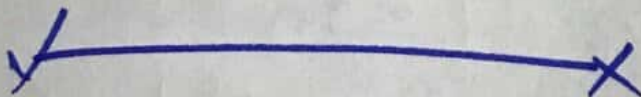
$$\begin{aligned}q_{n.s} &= \frac{q_{n.u}}{F.O.S} \\ &= \frac{733.2}{3} = 244.4 \text{ kN/m}^2\end{aligned}$$

$$\begin{aligned}q_s &= q_{n.s} + \bar{s} \\ &= 244.4 + (18 \times 1.6) \\ &= 273.2 \text{ kN/m}^2\end{aligned}$$

" Total safe load on rectangular footing: "

$$A \times q_s = (2 \times 3) \times 273.2$$

$$= 1639.2 \text{ kN}$$



Q3 a)

⑨

What is settlement. What are its type and explain in detail?

Ans:-

"Settlement:-"

It is defined as "When a soil deposit is loaded, deformation will occur due to change in stress - the total vertical downward deformation at the surface resulting from the load is called settlement -"

"Type of Settlement:-"

On the basis of movement of structure it is divided into two types:

- Total Settlement
- Differential Settlement

"Total Settlement:-"

→ Also known as uniform settlement -

→ When all the points settle with an equal amount, the settlement is known as uniform settlement -

→ This type of settlement is possible only under rigid foundation loaded with uniform pressure and resting on uniform soil deposit which is very rare possibility -

This type of settlement may not endanger the structure stability but generally affects the utility of the structure by jamming doors and damaging the utility lines. (10)

" Limitation for total Settlement:- "

The soil layer to which the load is to be transfer should be sufficient in bearing to resist the load which is to be applied on it - To spread the coming load over a large area -

• Differential Settlement:- "

- Differential settlement is more danger as compared with total or uniform settlement, because it cause more damage to a structure as compared to total settlement.
- When different parts of structure settle by different magnitude, the settlement is called differential settlement -
- If soil is granular, then differential settlement will be $\frac{2}{3}$ of the total maximum settlement -
- In case of cohesive soil, possible differential settlement is about $\frac{1}{3}$ of the maximum settlement

"Types of Differential Settlement:"

(11)

There are two types

of differential settlement:

- ① Tilt
- ② Angular Distortion.

Q3 b) "Given Data:-"

$$C_c = 0.31$$

$$P_1 = 130 \text{ kN/m}^2$$

$$e_0 = 1.02$$

$$P_2 = 170 \text{ kN/m}^2$$

$$H = 5 \text{ m}$$

"Required:-"

$$e_1 = ?$$

$$S_c = ?$$

"Solution:-"

$$C_c = \frac{\Delta e}{\log_{10} \left(\frac{P_2}{P_1} \right)}$$

$$= \frac{e_0 - e_1}{\log_{10} \left(\frac{P_2}{P_1} \right)}$$

$$0.31 = \frac{1.02 - e_1}{\log_{10} \left(\frac{170}{130} \right)}$$

$$e_1 = 0.983$$

(12)

$$\text{Now } s_c = \frac{H}{1+e_0} \times C_e \log_{10} \left(\frac{P_2}{P_1} \right)$$

$$= \frac{5}{1+1.02} \times 0.31 \log_{10} \left(\frac{170}{130} \right) \times 1000$$

$$= 2.47 \times 0.03611 \times 1000$$

$$s_c = 89.39 \text{ mm}$$

