

Assignment No 1  
ID No 7786  
Section A  
Ahmed Shahzad

## VENTURI FLUME :-

It is a critical-flow open flume with a constricted flow which causes a drop in hydraulic grade line, creating a critical depth. It is used in flow measurement of very large flow rates. ~~It~~ <sup>venturi meter</sup> is normally measured in millimeters, and venturi flume in meters.

Measurement of discharge with venturi flume requires two measurements, one upstream and one at the throat; if the flow passes in subcritical state through flume, a single measurement at throat is enough to measure the discharge.



Q2)

**Solution:-**

$$b = 3 \text{ m}$$

$$Q = 12 \text{ m}^3 \text{ s}^{-1}$$

**Discharge per unit width:-**

$$q = Q/b = 12/3 = 4 \text{ m}^2/\text{s}$$

For a rectangular channel:-

$$h_c = (q^2/g)^{1/3} = (4^2/9.8)^{1/3} = 1.177 \text{ m}$$

**Critical depth:- 1.18 m**

v) For Rectangular channel:-

$$E_c = \frac{3}{2} h_c = \frac{3}{2} \times 1.177 = 1.766 \text{ m}$$

Answer:- Minimum Specific energy = 1.77 m.

c)  $E > E_c$ , there are two possible depths for a given specific energy.

$$E = h + \frac{v^2}{2g} \quad \text{where } v = \frac{Q}{A} = \frac{q}{h}$$

$$E = h + \frac{q^2}{2gh^2}$$

Substituting values in m-sec unit.

$$4 = h + \frac{0.8155}{h^2}$$

arrange eq for "h"

$$h = 4 - \frac{0.8155}{h^2}$$

Iteration (from eq  $h=4$ ) gives  $h=3.948 \text{ m}$

For Super critical :-

$$h = \frac{4 - 0.8155}{h^2}$$

$$h = \frac{\sqrt{0.8155}}{\sqrt{4-h}}$$

~~Put  $h=0$~~   $h = 0.4814 \text{ m}$ .

Assignment No 2  
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Q1) Solution:-

To find the flow we have to find the  
Froude number:-

$$Fr = \frac{V}{\sqrt{gy}} = \frac{6 \text{ m/s}}{\sqrt{9.81 \times 0.1}}$$

$$Fr = 6.06 > 1$$

So the flow is supercritical

Alternate depth:-

As we know that

$$E = y + \frac{V^2}{2g}$$
$$= 0.1 + \frac{6^2}{2 \times 9.81} = 1.935 \text{ m}$$

The alternate depth for  $E = 1.935 \text{ m}$

$$y_{\text{alternate}} = 1.93 \text{ m}$$

\* == \*

Q2:- Solution:-

B Velocity = ?

$$\text{depth} = y_1 = 3 \text{ m}$$

$$\text{Elevation } \Delta x = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{downstep} = 15 \text{ cm} = 0.15 \text{ m}$$

As we know that

$$E_1 = y_1 + \frac{V_1^2}{2g}$$

$$E_1 = 3 + \frac{2^2}{2 \times 9.81}$$

$$E_1 = 3.20 \text{ m}$$

Now,  $E_2 = E_1 - \Delta Z$

$$E_2 = 3.2 - 0.6$$

$$E_2 = 2.60 \text{ m}$$

$$E_2 = y_2 + \frac{v^2}{2gy_2^2}$$

Arrange the eq for "y".

$$2.60 = y_2 + \frac{b^2}{2 \times 9.81 y_2^2}$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_2 - y_1$$

$$\Delta y = 2.24 - 3$$

$$\Delta y = 0.76 \text{ m}$$

So water surface drop is 0.6m

\* for a downward steep of 15cm or 0.15m

$$E_2 = E_1 - \Delta Z = 3.20 - (0.15)$$

$$E_2 = 3.35 \text{ m}$$

Now  $y_2 = 3.17 \text{ m}$

$$\Delta y = y_2 - y_1 = 3.17 - 3$$

$$\Delta y = 0.17 \text{ m}$$

So water surface rises 0.02m

The maximum upstep possible before effecting  
upstream water <sup>surface</sup> level <sup>is</sup> ~~surface~~ for

$$y_L = y_c$$

$$y_c = \sqrt[3]{\frac{q^2}{g}}$$

$$y_c = \sqrt[3]{\frac{6^2}{9.81}}$$

$$y_c = 1.54 \text{ m}$$

Assignment No 3  
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Data:-

$$y_1 = 3.6\text{m}, y_2 = 0.9\text{m}, b = 3.9\text{m}$$

SOLUTION:-

As we know that

$$E_1 = E_2$$

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g} \rightarrow \text{eq (i)}$$

$$\text{Also, } Q = A_1 V_1 = A_2 V_2$$

$$b_1 y_1 V_1 = b_2 y_2 V_2$$

$$y_1 V_1 = y_2 V_2$$

$$V_2 = y_1 / y_2 \times V_1$$

$$V_2 = 3.6 / 0.9 \times V_1$$

$$V_2 = 4 V_1$$

Putting value of  $V_2$  in eq (i)

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g}$$

$$3.6 + \frac{V_1^2}{2g} = 0.9 + \frac{4(V_1)^2}{2g}$$

$$\frac{V_1^2}{2g} - \frac{16V_1^2}{2g} = 0.9 - 3.6$$

$$\frac{V_1^2 - 16V_1^2}{2g} = -2.7$$

$$\sqrt{v_1^2} = \sqrt{\frac{2.7 \times 2(9.8)}{15}}$$

$v_1 = 1.879 \text{ m/sec} \Rightarrow$  put in eq<sup>n</sup>s we get.

$$v_2 = 4v_1 \Rightarrow v_2 = 4(1.879)$$

$$v_2 = 7.516 \text{ m/s}$$

As  $Q_1 = A_1 v_1 = b y_1 v_1$

$$= 3.9 \times 3.6 \times 1.879$$

$$Q_1 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 v_2 = b y_2 v_2$$

$$= 3.9 \times 0.9 \times 7.516$$

$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 v_2 = b y_2 v_2$$

$$= 3.9 \times 0.9 \times 7.516$$

$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

**Froude No at Upstream Side**

$$Fr_1 = \frac{v_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}} = 0.31 \text{ (sub critical flow)}$$

**Froude No at downstream Side:-**

$$Fr_2 = \frac{v_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}}$$

$$= 2.52 \text{ Super critical flow}$$