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Subject :- Advance Fluid Mechanics

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Deptt :- BE (civil)

Q No 1

(1)

(a) Drag:

A body which is wholly immersed in a homogenous fluid may be subjected to two kind of forces arising from relative motion b/w body and fluid these forces are termed as drag and lift. if the forces parallel to the motion then it is termed as drag force.

⇒ There are two components.

(i) Pressure Drag (FP):

It is equal to integrated of components in direction of motion of all pressure forces extended on surface of body.


$$FP = CP \int \frac{V^2}{2} A$$

where CP depends on shape.

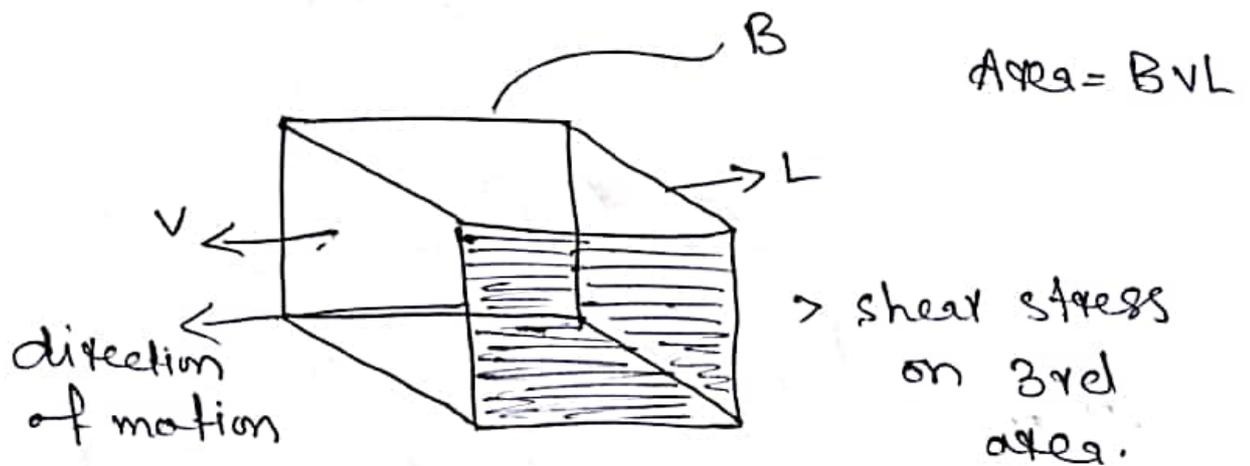
② Friction Drag (F_f)

it is equal to integration of components of shear stress along surface of body in direction of motion.

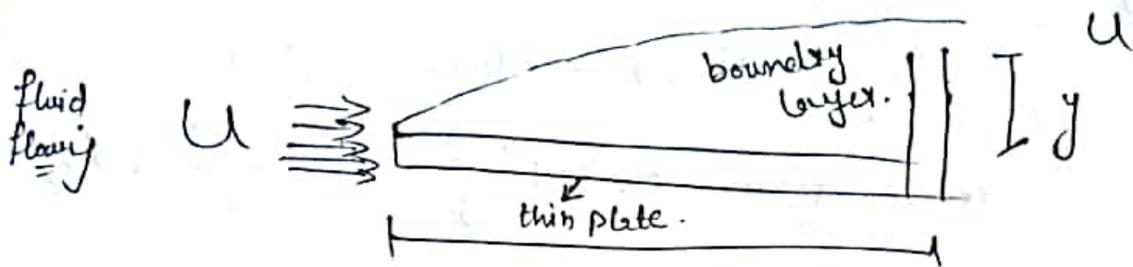
$$F_f = c_f \int \frac{v^2}{2} BL$$

shear stress

Fig.:

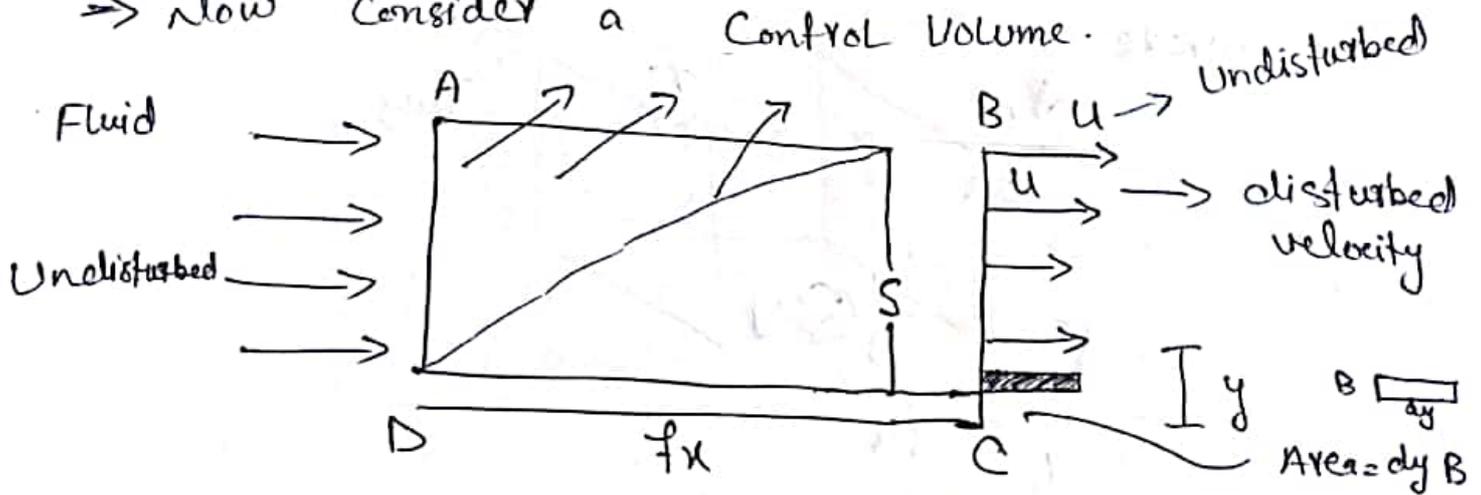


⇒ Friction Drag of Boundary Layer (3)



⇒ Fig shows growth of boundary layer along one side of smooth plate inside the fluid.

⇒ Now consider a Control Volume.



⇒ where S is thickness of boundary layer and U is Undisturbed velocity

Thus

$$-F_x = \text{drag} = (\text{rate in momentum in } x\text{-direction})$$

(4)

\Rightarrow (Leaving through BC + rate of momentum through AB) - rate of momentum entering through DA)

$$\Delta P = P_{out} - P_{in}$$

Thus according to momentum

$$\leq F = \frac{d}{dt}(P) = \frac{dmov}{dt}$$

where

$$\frac{dm}{dt} = \int \rho \quad \text{Thus}$$

$$F = \int \rho v$$

or

$$F = \int A \cdot v \cdot v$$

$$F = \int A v^2$$

$$DA \rightarrow \int U \cdot (UBS)$$

$$BC \rightarrow \int_B \int u^2 \cdot dy$$

(5)

$$AB \rightarrow \int_0^s u (UBS - B \int_0^s u \cdot dy)$$

Putting value

$$F_x = \int_B \int_0^s u (u - u) dy$$

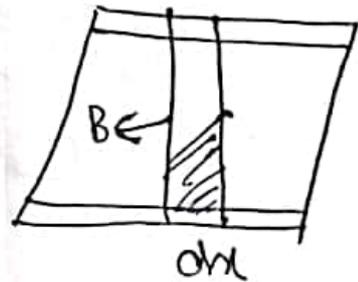
$$F_x = \int_B u^2 f \alpha$$

where α is function of boundary layer.

Now to find local wall shear stress

$$\tau_0 = \frac{dF_x}{B \cdot dx - \text{area}}$$

$$F_x = \int B u^2 f \alpha$$



$$\tau_0 = \int u^2 \alpha \frac{ds}{dx} \quad \text{in general}$$

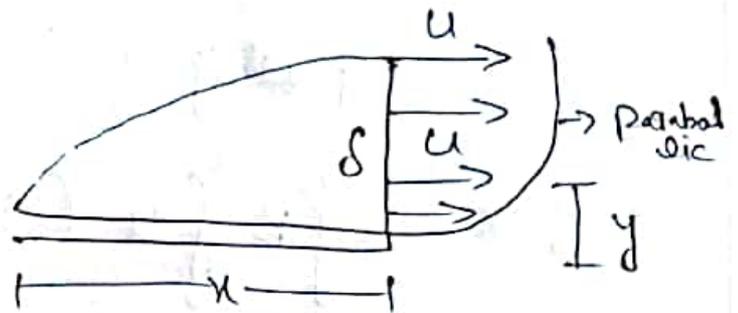
equation of shear stress.

(6)

→ Laminar boundary layer :-

$$\frac{u}{V} = F\left(\frac{y}{\delta}\right)$$

Assume



$$\eta = \frac{y}{\delta} \quad \text{or} \quad y = \eta \delta$$

Thus

$$\frac{u}{V} = f(\eta) \quad \text{or} \quad u = U f(\eta)$$

In case of laminar flow

$$\tau_0 = \mu \left(\frac{du}{dy} \right)$$

$$= \frac{\mu}{\delta} \left(\frac{du}{d\eta} \right) = \frac{\mu U}{\delta} \left[\frac{df}{d\eta}(\eta) \right]$$

Solving the Eq.

(7)

$$Z_0 = \frac{u_{UB}}{S} \rightarrow (1)$$

As general Equation is $Z_0 = \int u^2 \alpha \frac{ds}{dx}$

Equating both equation.

$$\frac{u_{UB}}{S} = \int u^2 \alpha \frac{ds}{dx}$$

or

$$S ds = \frac{u_{UB}}{\int u^2 \alpha} dx$$

Integrating the equation.

$$\frac{S^2}{2} = \frac{u_{UB}}{\int u^2 \alpha} x + C$$

Now at $x=0$, $S=0$ Thus $C=0$

$$\frac{S^2}{2} = \frac{u_{UB}}{\int u^2 \alpha} x$$

(8)

or

$$\delta = \frac{\sqrt{2 \mu B}}{\rho U \alpha} x \quad \text{or} \quad \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}}$$

Multiplying and dividing by "x"

$$\delta = \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}} \cdot \frac{x}{\sqrt{x} \cdot \sqrt{x}}$$

where

$$\alpha = 0.135$$

$$B = 1.63$$

$$R_x = \frac{\rho U x}{\mu}$$

$$\delta = \frac{4.91}{\sqrt{R_x}} \cdot x \quad \text{or} \quad \frac{\delta}{x} = \frac{4.91}{\sqrt{R_x}}$$

Now

$$Z_0 = \frac{\mu U B}{\delta}$$

Thus putting value

$$Z_0 = 0.332 \frac{\mu U}{x} \sqrt{R_x}$$

where R_x is local Reynolds number.

(9)

Now

$$F_g = B \int_0^L \frac{\tau_0 dx}{\text{stress}}$$

Putting values.

$$F_g = 0.664 B \sqrt{\rho \mu L V^3}$$

As general equation is

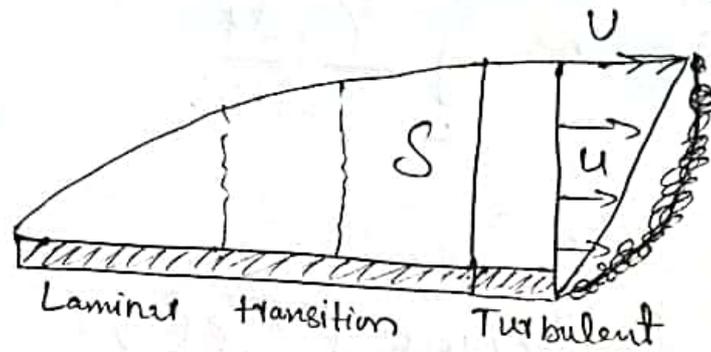
$$F_f = C_f \rho \frac{V^2}{2} BL \rightarrow \text{Equating both equations}$$

$$C_f = 1.328 \sqrt{\frac{\mu}{\rho L V}} = \frac{1.328}{\sqrt{R}}$$

————— x —————

(10)

TURBULANT BOUNDARY LAYER:



resistance
is less
so curve
become
straight

Fig show that velocity distribution in turbulent boundary layer shows a much steeper gradient near wall and flatter through out remaining layer.

⇒ To shear stress is greater in turbulent than in laminar layer.

As we have.

$$\tau_0 = f \frac{\rho v^2}{8}$$

where v denotes average velocity of pipe.

(11)

→ now we have obtained an approximate relation b/w V and U by using pipe factor Equation of

$$\frac{V}{U_{\max}} = \frac{1}{1 + 1.33\sqrt{f}}$$

Using friction factor of 0.028 from chart which is middle critical value.

So

$$U = 1.235V$$

Now we have

$$\tau_0 = f \int \frac{v^2}{8}$$

As we know

$$f = \frac{0.316}{R^{0.25}}$$

Thus

$$\tau_0 = \frac{0.316}{\left(\frac{DV}{\nu}\right)^{1/4}} \int \frac{v^2}{8}$$

(12)

where

$$V = \frac{U}{1.235} \quad \text{thus}$$

$$\tau_0 = \frac{0.316}{\left(\frac{\rho}{\nu} \left(\frac{U}{1.235}\right)\right)^{1/4}} \cdot \frac{\rho}{8} \left(\frac{U}{1.235}\right)^2$$

$$\leq D = 28$$

thus

$$\tau_0 = \frac{0.0238 U^2}{\left(\frac{\rho U}{\nu}\right)^{1/4}}$$

As we have

$$\tau_0 = \rho U^2 \alpha \frac{ds}{dx}$$

Equating both and integrating for boundary condition of $x=0$, $s=0$

(13)

Thus

$$S = \left(\frac{0.0287}{\alpha} \right)^{4/5} \left(\frac{v}{Ux} \right)^{1/5} x.$$

$$\text{For } \alpha = 0.0972$$

$$\frac{S}{x} = \frac{0.377}{(Rx)^{1/5}}$$

Putting values in equation.

$$z_0 = 0.0587 \int \frac{U^2}{2} \left(\frac{U}{Vx} \right)^{1/5}$$

Now

$$F_f = B \int_0^2 z_0 dx$$

$$F_f = 0.0735 \int \frac{U^2}{2} \left(\frac{v}{UL} \right) BL$$

$$\text{As } F_f = C_f \int \frac{v^2}{2} BL.$$

Equating Both:

$$C_f = \frac{0.0735}{R^{1/5}}$$

R is less than
 10^7 for
 $500,000 < R < 10^7$

(14)

For $R > 10^7$

$$Cf = \frac{0.455}{(\log R)^{2.58}}$$

As

X

~~11~~

①

Q No 1
Part (B)

As specific Energy

$$E = y + \frac{v^2}{2g}$$

the flow Q per unit width b can be expressed as

$$q = \frac{Q}{b}$$

Now average velocity will be

$$V = \frac{Q}{A} = \frac{qb}{by} = \frac{q}{y}$$

Thus

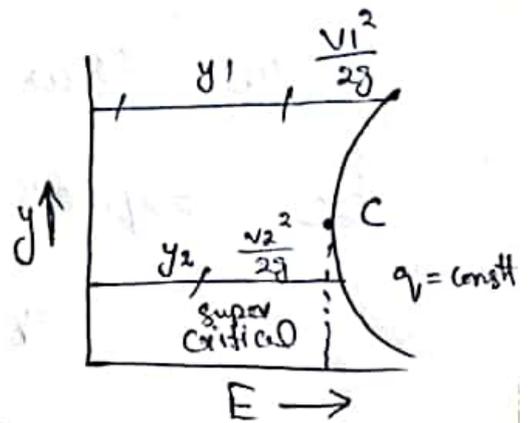
$$E = y + \frac{v^2}{2g} \Rightarrow y + \frac{1}{2g} \left(\frac{q^2}{y^2} \right)$$

$$(E - y) = \frac{1}{2g} \left(\frac{q^2}{y^2} \right) \quad \text{or} \quad (E - y)y^2 = \frac{q^2}{2g}$$

(2)

This plot of E vs y will be parabolic. For particular q , there will be two kind of possible values of y , for a given E .

The Equation is cubic with three roots, with third root being negative point C represents dividing point b/w two regime of flow



Thus for given q , ξ value of E is minimum ξ Flow at that point is critical flow. Depth of flow at that point is critical depth y_c ξ velocity at that point is critical velocity v_c .

Thus

(3)

$$E = y + \frac{1}{2g} \left(\frac{q^2}{y^2} \right)$$

For minimum specific energy $\frac{dE}{dy} = 0$

Thus

$$\frac{dE}{dy} = 1 - \frac{2}{2g} \left(\frac{q^2}{y^3} \right) = 0$$

$$\Rightarrow \frac{q^2}{gy^3} = 1 \Rightarrow q^2 = gy^3$$

$$\frac{q^2}{g} = y^3 \Rightarrow \left(\frac{q^2}{g} \right)^{1/3} = y_{cr}$$

Now

$$q^2 = gy^3$$

ξ

$$q = Vy \Rightarrow v^2 y^2 = gy^3$$

or

$$v^2 = gy$$

or

$$V_{cr} = \sqrt{gy_{cr}}$$

— {Question 02} —

* Given Data :-

Water flow at rate, $Q = 3.5 \text{ m}^3/\text{s}$

Bed slope, $S_0 = 0.0008$

$n = 0.0219$

width of bed is student I-D = ~~7.690~~ 7.690 m

* Required :-

Depth of rectangular channel = ?

Critical depth, $y_c = ?$

Critical velocity $V_c = ?$

Flow is critical or sub-critical = ?

* Solution :-

$$Q = \left(\frac{1}{n} R_n^{2/3} S_0^{1/2} \right) A \quad \text{--- (1)}$$

$$\text{Area} = 7.690 \times d$$

$$\text{Parameter} = d + 7.690 + d$$

$$\text{Hydraulic Radius} = R_n = \frac{\text{Area}}{\text{Parameter}} = \frac{7.690 \times d}{2d + 7.690}$$

we know that

$$Q = \left(\frac{1}{n} R_n^{2/3} S_0^{1/2} \right) A$$

Putting values

$$3.5 = \left(\frac{1}{0.0219} \times \left(\frac{7.690 \times d}{2d + 7.690} \right)^{2/3} \times (0.0008)^{1/2} \right)^2$$

$$\frac{3.5 \times 0.219}{\sqrt{0.0008}} = \left(\frac{7.690 \times d}{2d + 7.690} \right)^{2/3}$$

$$(2.57)^{3/2} = \frac{7.690 d}{2d + 7.690} \times 7.690 d$$

$$4.461 \cdot (2d + 7.690) = 59.13 d^2$$

$$8.92d + 34.30 = 59.13d^2$$

$$59.13d^2 - 8.92d - 34.21 = 0$$

$$d = 0.84 \text{ m}$$

3

* So depth of the channel is 0.840 m

⇒ As

$$v = \frac{Q}{b}$$

$$v = \frac{3.5}{7.690} = 0.4551$$

⇒ For critical depth :-

$$y_{cr} = \left(\frac{v^2}{g} \right)^{1/3}$$

$$y_{cr} = \left(\frac{0.4551^2}{9.81} \right)^{1/3}$$

$$\boxed{y_{cr} = 0.276 \text{ m}}$$

Now critical velocity

$$V_{cr} = \sqrt{y_{cr} \times g}$$

$$V_{cr} = \sqrt{(0.276)(9.81)}$$

$$\boxed{V_{cr} = 1.645 \text{ m/s}}$$

$$V = \frac{Q}{A}$$

$$V = \frac{3.5}{7.690 \times 0.840}$$

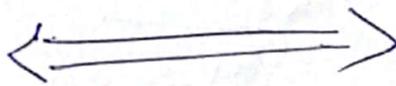
$$V = 0.542 \text{ m/s}$$

$$y = 0.840 \text{ m}, \quad y_{cr} = 0.276 \text{ m}$$

$$V = 0.542 \text{ m/s}, \quad V_{cr} = 1.645 \text{ m/s}$$

$$\text{As } y > y_{cr} \text{ \& } V < V_{cr}$$

~~So~~ \Rightarrow So flow is subcritical.



— {Question 03} —

Given data: width of smooth plate $B = 200 \text{ mm}$
 $B = 0.2 \text{ m}$

length of plate $L = 800 \text{ mm}$
 $L = 0.8 \text{ m}$

Specific gravity of oil $S = 0.89$

Undisturbed velocity $v = 5 \text{ m/s}$

Kinematic viscosity $\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$

Required data:

Friction drag $F_f = ?$

Solution:

As we know,
The friction drag is given by the equation,

$$F_f = C_f \rho \frac{v^2}{2} BL \quad \text{--- (1)}$$

where C_f depends on viscosity.

Reynold's number is given by

$$R = \frac{\rho v L}{\mu} = \frac{v L}{\nu} \quad \text{where } \nu = \frac{\mu}{\rho}$$

Putting values we get

$$R = \frac{5 \text{ m/s} \times 0.8 \text{ m}}{0.93 \times 10^{-4} \text{ m}^2/\text{s}}$$

$$R = 43010.75 \quad \text{which is less than } 500,000.$$

So,

$$C_f = \frac{1.328}{\sqrt{R}} = \frac{1.328}{\sqrt{43011}} = 0.00640$$

Now:

$$\text{As Specific Gravity} = \frac{\rho_{\text{soil}}}{\rho_{\text{water}}}$$

$$\rho_{\text{soil}} = \text{Specific Gravity} \times \rho_{\text{water}}$$

$$\rho_{\text{soil}} = 0.89 \times 1000$$

$$\rho_{\text{soil}} = 890$$

Putting values in equation (1), we get

$$F_f = 0.00640 \times 0.89 \times 1000 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = 11.392 \text{ N}$$

So, the friction drag on one side of smooth plate is '11.392 N.'

