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Section:

B

Depth:

BE(C)

Subject:

Structure Analysis

Submitted to:

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Question no 012

Write a detail note in your own words on different types of load that different type of structure are designed to support throughout its life. Elaborate with examples.

Loads

It becomes necessary to determine the loads the structures must support often, it is anticipation of the various load that will be imposed on the structure that will be chosen for design. For example high-rise structure must endure large lateral loading caused by wind or so shear walls and tubular frame system are selected whereas building located in area prone to earthquakes must be designed having ductile frames and connection

OR

Load are defined as, loads imposed on a completed or temporary structure during and as a result of the construction process. Any external force or couples moment is said to be load when these are acting on the member during its functionality.

Types of loads

The types of load acting on structures for buildings and other structures can be broadly classified into vertical loads and longitudinal loads.

The vertical loads consist of dead load, live load and impact load.

The horizontal loads comprises of wind load and earthquake load.

The longitudinal loads i.e tractive loads and braking forces are considered in special case of designed ~~load~~ bridge, gantry girders etc.

In a construction of building two major factors considered are safety

and economy. They are adjudged and taken higher than economy is affected. If economy is considered and loads are taken lesser than the safety is compromised.

Types of loads action on a structure are;

- 1) Dead loads
- 2) Imposed loads (Live loads)
- 3) Wind loads
- 4) Snow loads
- 5) Earthquake loads
- 6) Special loads

## 1) Dead Loads (DL)

The first vertical load that is considered is dead load. Dead loads are permanent or stationary loads which are transferred to structure throughout the life span. Dead load is primarily due to self weight of structural members, permanent partition walls, fixed permanent equipments and weight of different

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materials. It majorly consists of the weights of roof, beams, walls and column etc. which are otherwise the permanent parts of the buildings.

## 2) Imposed Loads or Live Loads

The second vertical load that is considered in the design of a structure is imposed load or live loads. Live loads are either movable or moving loads without any acceleration or impact. These loads are assumed to be produced by the intended load use or occupancy of the building including weights of movable partitions or furniture etc.

Live loads keep on changing from time to time. These loads are to be suitably assumed by the designer. It is one of the major loads in the design. The minimum values of live loads to be assumed are given in IS 875 (part 2) - 1987. It depends upon the intended use of the building.

### 3) Wind Loads

Wind load is primarily horizontal load caused by the movement of air relative to earth. Wind load is required to be considered in structural design especially when the height of the building exceeds two times the dimensions transverse to the exposed wind surface.

For low rise building say up to four to five stories, the wind load is not critical because the movement of resistance provided by the continuity of floor system to column connection and wall provided between column are sufficient to accommodate the effect of these forces. Further in limit state method the factor for designed load is reduced to  $1.2 (DL + LL + WL)$  when wind is considered as against the factor of  $1.5 (DL + LL)$  when wind is not considered.

### 4) Snow Loads

Snow loads constitute to the vertical loads in the building. But these types of loads are considered

Only in the Snow fall places. The IS 875(part 4)-1987 deal with the Snow loads on roofs of the building.

The minimum Snow load on a roof area or any other area above ground which is subjected to Snow accumulation is obtained by the expression

$$S = \mu S_0$$

Where  $S$  = Design load on plane area of the roof

$\mu$  = Shape Coefficient

$S_0$  = Ground Snow load

### 5) Earthquake Loads:

Earthquake Forces constitute to both vertical and horizontal forces on the building. The total vibration caused by earthquake may be resolved into three mutually perp direction, usually taken as vertical and two horizontal direction.

The movement in vertical direction do not cause forces in Superstructure to any significant extent. But the horizontal movement of the building at the time

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of earthquake is to be considered while designing.

## b) Other loads and Effects acting on Structures

As per the Clause 19.6 of IS 456-2000, in addition to above load discussed, account shall be taken of the following forces and effects if they are liable to affect materially the safety and serviceability of the structure.

- a) Foundation movement (IS 904)
- b) Soil and fluid pressure (IS 1904) (875)
- c) Elastic & Shortening
- d) Vibration
- e) Fatigue
- f) Impact (IS 875)
- g) Erection loads (IS 875)
- h) Stress Concentration effect due to point load and the like.



# Different types of Structures are designed to support loads

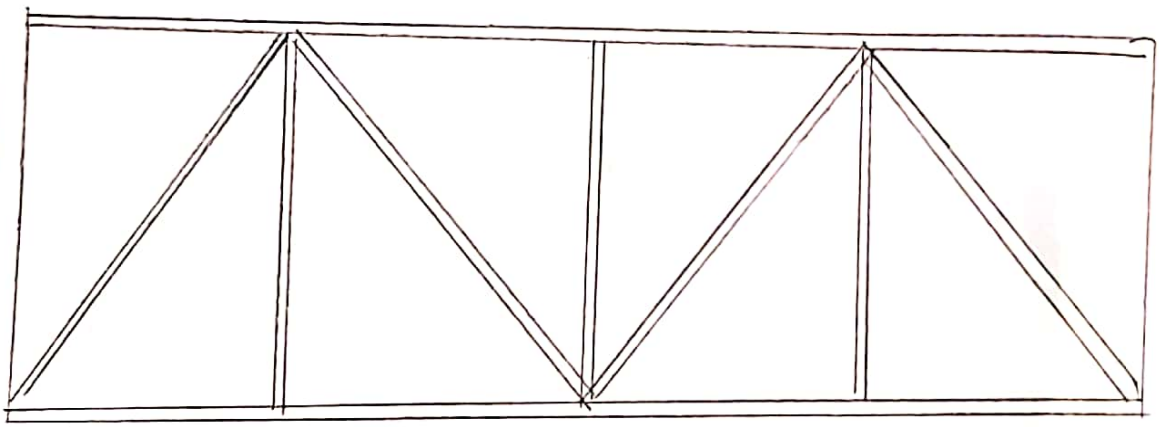
## Column

Members that are generally vertical and resist axial compression load are referred to as columns. Tubes and wide-flange cross-section are often used for metal columns, and circular and square cross sections with reinforcing rods are used for those made of concrete. Occasionally columns are subjected to both an axial load and a bending moment. These members are referred to as beam



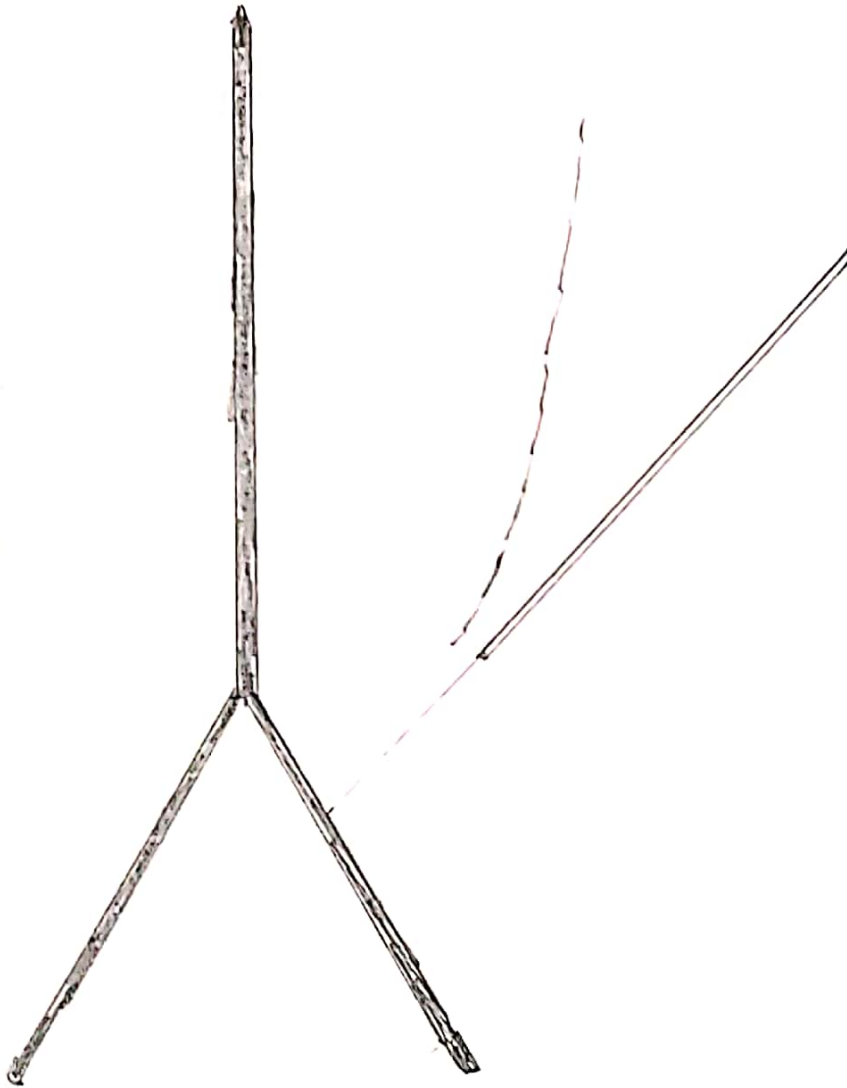
# Trusses

When the span of a structure is required to be large and its depth is not an important criterion for design, a truss may be selected. Planar trusses are composed of members that lie in the same plane and are frequently used for bridge and roof support whereas space trusses have members extending in three dimensional and are suitable for derricks and towers.



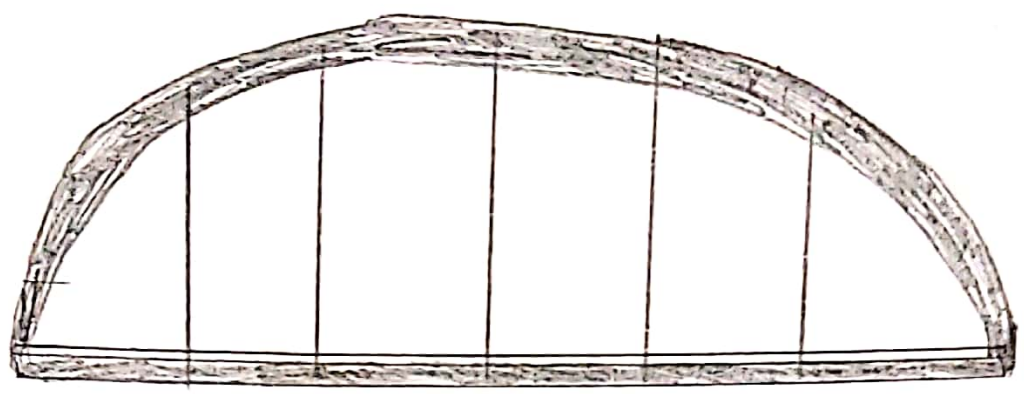
# Cables

Cables are usually flexible and carry their load in tension. They are commonly used to support bridge, and building roofs. When used for these purposes, the cable has an advantages over the beam and the truss, especially spans that are greater than 150ft (46m).



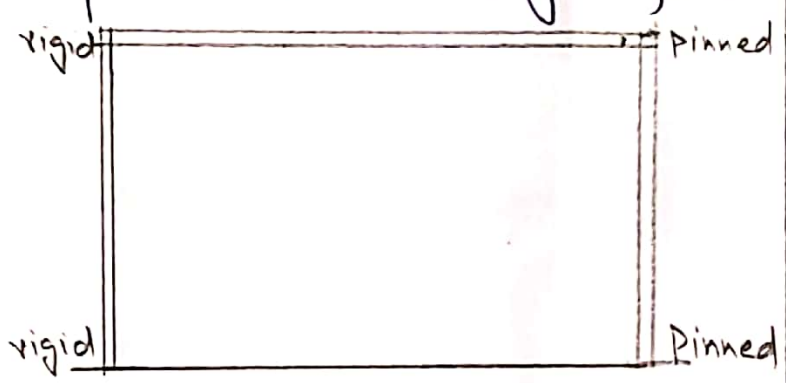
## Arch

Arch achieves its strength in compression, since it has a reverse curvature to that of the cable. The arch must be rigid, however, in order to maintain its shape and this results in secondary loadings involving shear and moment, which must be considered in its design. Arch are frequent used in bridge structures, dome roofs, and for opening in masonry walls.



## Frames

Frames are often used in building and are composed of beam and columns that are either pin or fixed connected. Like frame extends in two or three dimension. The loading on a frame cause bending of its members and if it is a rigid joint connection.



## Surface Structure

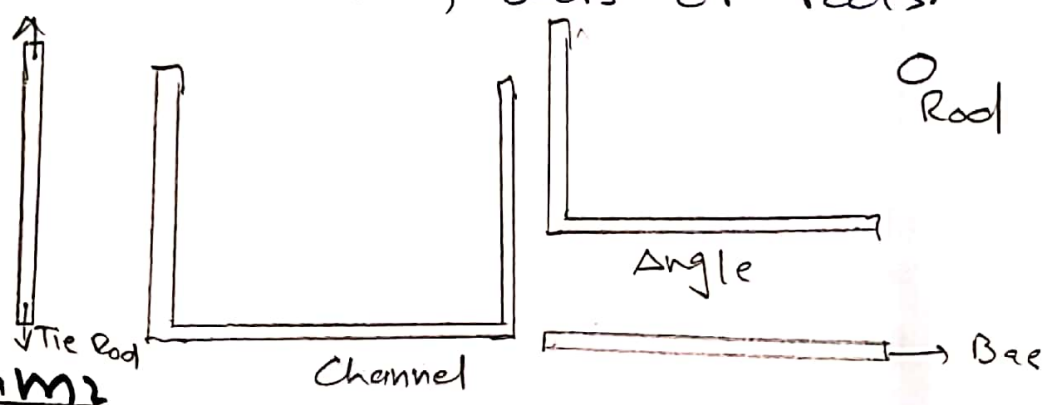
A Surface Structure is made from a material having a very small thickness compared to its other dimensions. Sometime this material is

Very flexible and can take the form of a tent or air-inflated structure. In both cases the material act as a membrane that is subjected to pure tension.



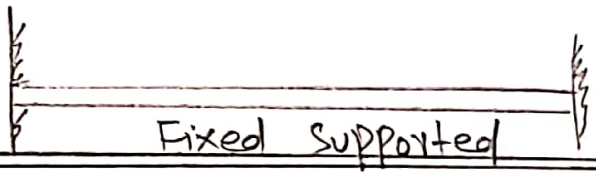
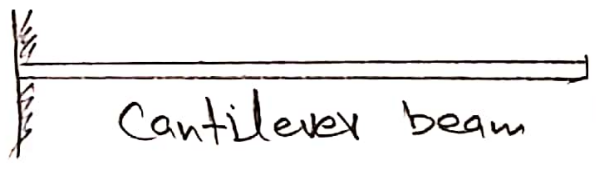
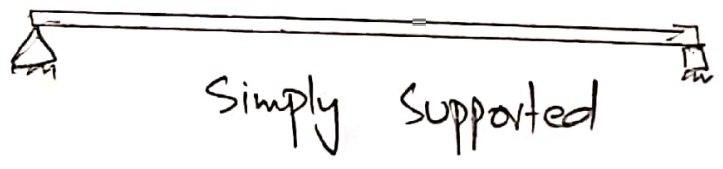
### Tie rods

Consist of tensile force. These members denoted, bars or rods.



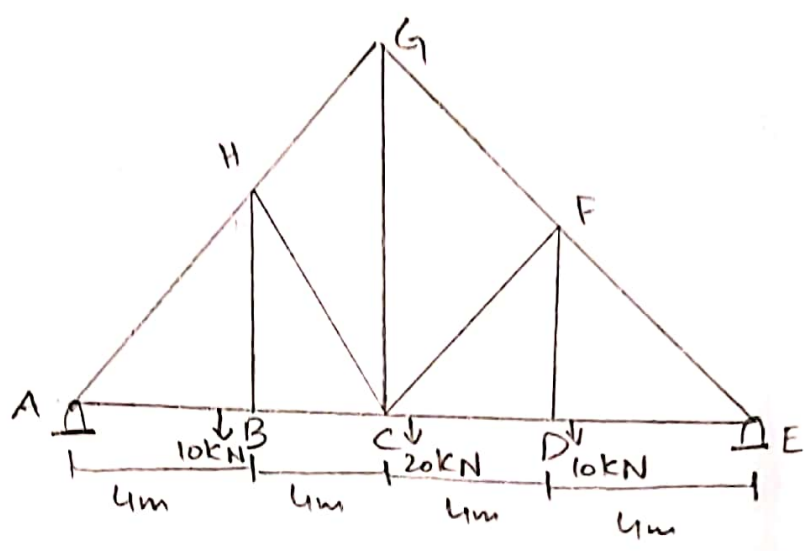
### Beams

They are horizontal members and supports vertical loads. It resist bending moment, short carry large loads.



Question No 022

Determine the force in each member of the truss. state if the members are in Tension or Compression. Assume all members are pin connected?



Required

Force in each members = ?

Solution

$$\sum F_y = 0 \quad \uparrow + \downarrow -$$

$$R_A + R_E = 40 \quad \rightarrow (A)$$

$$\sum M_A = 0 \quad \curvearrowright -$$

$$R_E(6) + 10(12) + 20(8) + 10(4) = 0$$

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$$R_E = \frac{320}{16} = 20 \text{ kN} \longrightarrow (B)$$

Put eqn B value in eqn A

$$R_A = 40 - 20$$

$$R_A = 20 \text{ kN}$$

Now determine force in each members.

Joint A

$$\sum F_y = 0;$$

$$-\frac{3}{5} (F_{AH}) + 20 \text{ kN} = 0$$

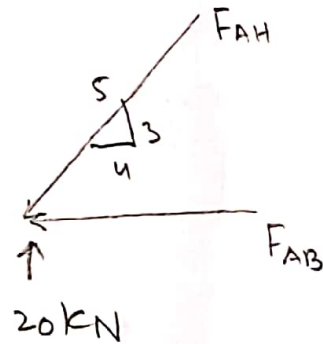
$$\Rightarrow -0.6 (F_{AH}) = -20 \text{ kN}$$

$$F_{AH} = 33.33 \text{ kN} (3)$$

$$\sum F_x = 0;$$

$$-\frac{4}{5} (33.33) + F_{AB} = 0$$

$$F_{AB} = 26.66 \text{ kN} (4)$$



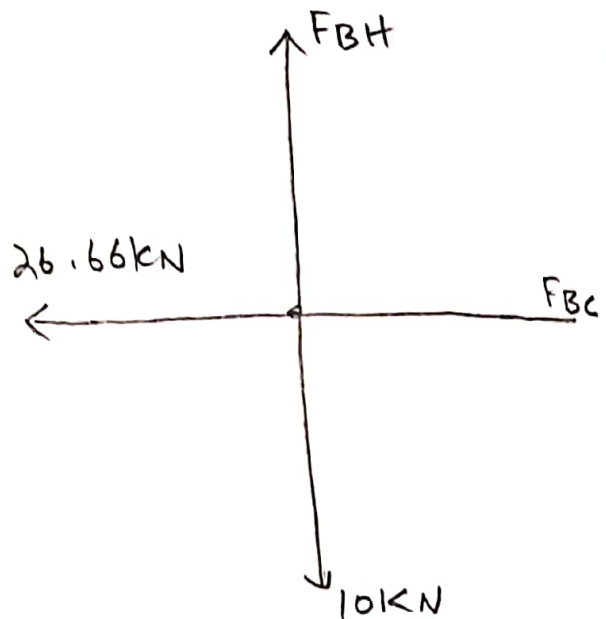
Joint B

$$\sum f_x = 0;$$

$$F_{BC} = 26.66 \text{ kN (T)}$$

$$\sum f_y = 0;$$

$$F_{BH} = 10 \text{ kN (T)}$$

Joint H

$$\sum F_y = 0;$$

$$\frac{3}{5}(33.33) - 10 \text{ kN} + \frac{3}{5}(F_{HC}) - \frac{3}{5}(F_{HG}) \rightarrow \text{a}$$

$$\sum F_x = 0;$$

$$\frac{4}{5}(33.33) - \frac{4}{5}(F_{HC}) - \frac{4}{5}(F_{HG}) \rightarrow \text{b}$$

So now solving eq a & b

$$19.98 - 10 + 0.6 F_{HC} - 0.6 F_{HG} = 0 \rightarrow \text{c}$$

$$26.66 - 0.8 F_{HC} - 0.8 F_{HG} = 0 \rightarrow \text{d}$$

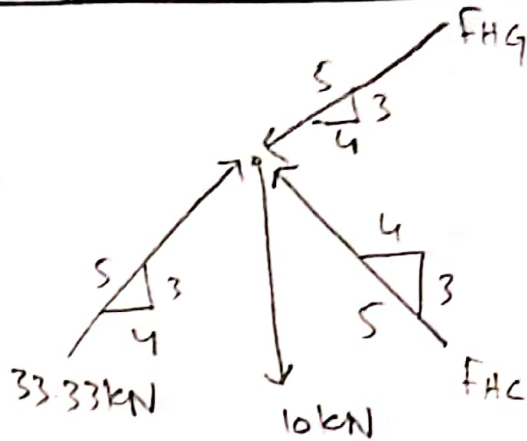
Now,

Multiplying eq c by 1.34 and then add with eq d we get;



$$F_{HG} = 25 \text{ KN (3)}$$

$$F_{HC} = 8.34 \text{ KN (3)}$$



### Joint G<sub>2</sub>

$$\sum F_x = 0;$$

$$\frac{4}{5}(25) - \frac{4}{5}(F_{GF}) = 0$$

$$F_{GF} = 25 \text{ KN (3)}$$

$$\sum F_y = 0;$$

$$\frac{3}{5}(25) + \frac{3}{5}(25) - F_{GC} = 0$$

$$F_{GC} = 30 \text{ KN (3)}$$

Due to symmetrical loading of structure we get,

$$F_{AB} = F_{ED} = 26.66 \text{ KN (4)}$$

$$F_{BC} = F_{DC} = 26.66 \text{ KN (4)}$$

$$F_{BH} = F_{DF} = 10 \text{ KN (2)}$$

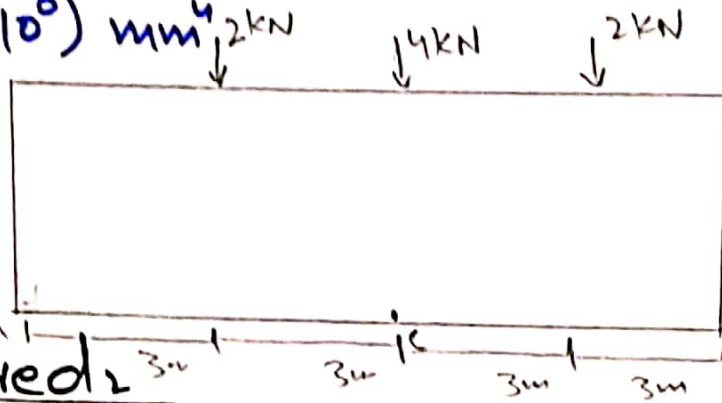
$$F_{HG} = F_{GF} = 25 \text{ KN (3)}$$

$$F_{HC} = F_{CC} = 8.34 \text{ KN (3)}$$

$$F_{AH} = F_{EF} = 33.34 \text{ KN (3)}$$

## Question No 032

Determine the Slope at A and displacement at C of the beam in the figure by a) Moment of theorem and Take  $E = 200 \text{ GPa}$ ,  
 $I = 6 (10^6) \text{ mm}^4$



Required:

Determine Slope at point 'A' and displacement at C using moment Area Theorem.

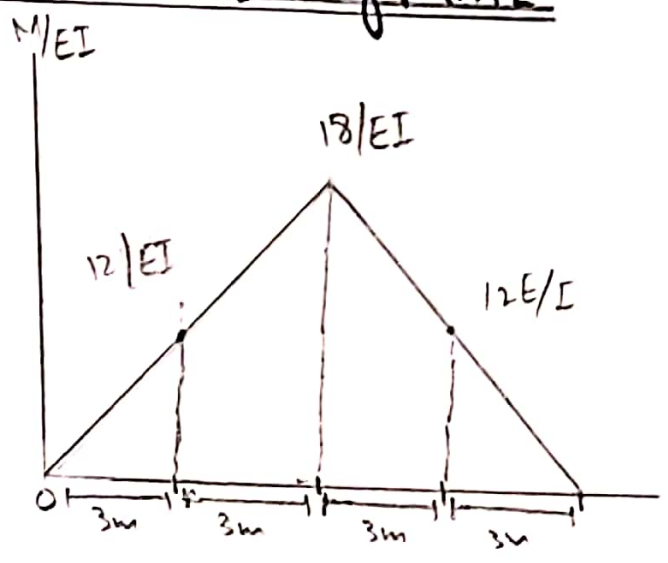
Given Data:

$$E = 200 \text{ GPa}$$

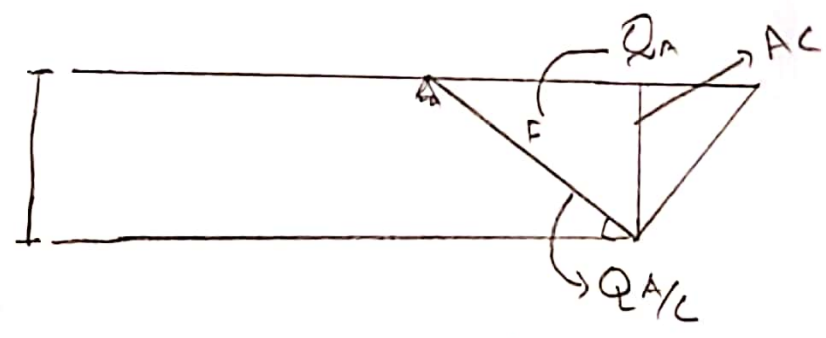
$$I = 6 \times 10^6 \text{ mm}^4$$

Finding out M/EI diagram of elastic curve

# Moment Diagram



# Elastic Curve



$$Q_{A/L} = \frac{1}{2} \left( \frac{12}{EI} \right) (3) + \left( \frac{12}{EI} \right) (3) + \frac{1}{2} \left( \frac{6}{EI} \right) (3)$$

$$Q_{A/L} = \left( \frac{18}{EI} \right) + \left( \frac{36}{EI} \right) + \left( \frac{9}{EI} \right)$$

$$Q_{A/C} = \left( \frac{63}{EI} \right)$$

$$Q_{A/C} = \frac{63}{(200 \times 10^6)(6 \times 10^6)(1000)^{-4}}$$

$$Q_{A/C} = 0.0525 \text{ radian}$$

$$Q_{A/C} = 0.0525 \text{ radian}$$

$$t_{A/C} = \left[ \frac{1}{2} \left( \frac{12}{EI} \right) (3) \left( \frac{2}{3} \right) (3) \right] + \left[ \frac{12}{EI} (3) \left( 3 + \frac{1}{2}(3) \right) \right] + \left[ \frac{1}{2} \left( \frac{6}{EI} (3) \right) \right] \left( 3 + \frac{2}{3}(3) \right)$$

$$t_{A/C} = 0.202m$$

$$\Delta_C = t_{A/C} = 0.202m$$

So, we get,

$$\Delta_C = t_{A/C} = 0.202m$$

$$\Delta_C = 202mm$$