

Kashif Khan

ID = 7846

Subject : highway & traffic

Semester : 6th

Section : "B"

Instructor : Abdul Farhan

Date : 22-6-20

Q1) What is the difference b/w flexible and rigid pavement?

Flexible pavement	Rigid pavement
-------------------	----------------

1) Bitumen is used as a binder in flexible pavement	Cement is used as a binder in rigid pavement
-----------------------------------------------------	----------------------------------------------

2) Deformation in the subgrade is transferred to the upper layers.	Deformation in the subgrade is not transferred to subsequent layers.
--------------------------------------------------------------------	----------------------------------------------------------------------

3) Load is transferred by grain to grain contact	⇒ No such phenomenon of grain to grain load transfer exists.
--------------------------------------------------	--------------------------------------------------------------

4) Have low life span usually 10-15 years.	Life is more as compare to flexible usually 30+ years.
--------------------------------------------	--------------------------------------------------------

5) Road can be used for traffic within 24 hours.	Road cannot be used until 14 days of curing.
--------------------------------------------------	----------------------------------------------

(b) What are the advantages of water bound over wet mix macadam?

→ The main advantages of wet mix macadam over water-bound macadam is that it is composed of a well graded mixture. This insures good interlock and high stability.

→ Addition of water while mixing facilitates the handling of the mixture. The operation layer laying is much simpler than that of water bound macadam, where the screening and binding material have to be added in stages and forced into voids. If a crusher-run material is used there is no possibility of plastic fines entering into the mixture.

→ The compaction is greatly facilitated by moisture added which lubricates the individual particles.

→ The aggregates for which wet mix macadam will have to be crushed run. Whereas the aggregates for water-bound macadam are generally hand broken.

c What is the difference b/w asphalt and bitumen?

Bitumen	Asphalt
---------	---------

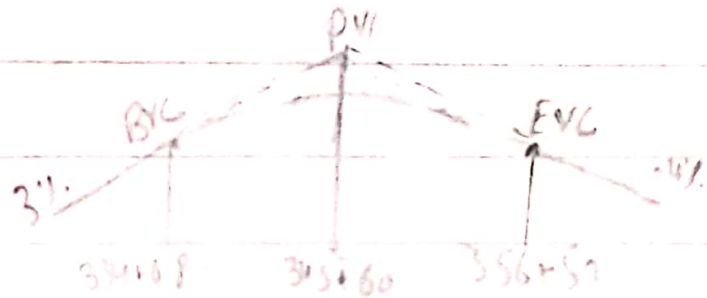
→ A class of black or dark-coloured cementation substance natural or manufactured composed principally of high molecular weight hydrocarbon found in tars, pitches are not typical	→ A dark brown to black cementitious material in which the natural or manufactured predominating constituents are bitumen which occur in nature or are obtained in fractional distillation along with certain mineral matter.
------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------

(4)

→ in some literature Bitumen is actually the liquid binder that holds asphalt together

→ Asphalt is generally used as a term to refer to the combination of bitumen and gravel specifically for road construction.

Q no 02
 A crest vertical curve joining a +3 Percent and +4 Percent grade is to be designed for 75 mph. If the tangents intersect at station (345+60,00) at an elevation of 250', determine the station and elevation of BVC and EVC. Also calculate the elevations of intermediate points on the curve at the whole stations.



Solution \Rightarrow

For a design speed of 75 mph,

$K = 312$ from table 15.5.

$$\text{minimum length} = 312 \times [3 - (-4)] = 2184'$$

$$\text{Station of BVC} = (345 + 60) - \left(\frac{21 + 84}{2}\right)$$

$$= 334 + 68$$

$$\text{Station of EVC} = (334 + 68) + (21 + 84) =$$

$$= 356 + 52$$

$$\text{Elevation of BVC} = 250 - \left(0.03 \times \frac{2184}{2}\right) = 217.24 \text{ ft.}$$

Q No. 3

A flexible pavement for an urban interstate highway is to be designed using 1993

- Ans Resilient modulus of Asphalt concrete at 68°F $450,000 \text{ lb/in}^2$
- CBR value of base course material 100 M_r $31,000 \text{ lb/in}^2$
- CBR value of sub base course material 22 , M_r $13,500 \text{ lb/in}^2$
- CBR value of subgrade material 6 M_r of subgrade $8 \times 15,00 \text{ lb/in}^2$
 $= 9000 \text{ lb/in}^2$

Flexible Pavement Design :->

- Reliability level (R) = 99%
- standard deviation $S_o = 0.49$.
- Initial Serviceability index $P_i = 4.5$
- Terminal Serviceability Index $P_t = 2.5$

$$\Delta PSI = P_i - P_t =$$
$$= 4.5 - 2.5 = 2.0.$$

Step - 1

Draw a line joining the reliability level of 99% and the overall standard deviation

So of 0.49 and extend this line to intersect the first TL at point A.

Finding the value of S_{mi} re. Di.

Step: 02

Draw a line joining point A to the ESAL 2×10^6 and this line to intersect the second TL line at point B.

Step: 3

Draw a line joining point B and resilient modulus (M_r) of base course and extend this line to intersect the design serviceability loss chart at point C.

Step : 04

- Draw a horizontal line from point C to intersect the design service life loss (PSI) curve at Point D:

$$\Delta PSI = P_i - P_f = 4.5 - 2.5 = 2.0$$

So the structure number required to protect the base course and to find the thickness D_i of surface course is 2.6

Step : 06

Determine the appropriate structure layer coefficient for each construction material, Resilient value of asphalt = 450,000 lb/in², therefore $a_i = 0.44$

Thickness of surface course D_i :

$$D_i = \frac{SN_i}{a_i}$$

$$\therefore SN_i = 2.6$$

$$a_i = 0.44$$

$$= \frac{2.6}{0.44} = 5.9''$$

900
• Thickness should be taken to the nearest 0.5"

So thickness of surface course is 6"

$$\begin{aligned} SN_1 &= 0.1 \times 21 \\ &= 2.1 \end{aligned}$$

$$SN_1 = 2.64$$

Finding SN_2 and D_2 (Base course).

$$\begin{aligned} D_2 &= (SN_2 - SN_1) / a_2 m^2 \\ &= (3.8 - 2.64) 0.14 \times 0.80 \end{aligned}$$

$$D_2 = 10.36$$

Use

12"

So thickness of base course 12"

$$SN_2 = 0.14 \times 0.80 \times 12 + SN_1$$

$$SN_2 = 1.34 + 2.64$$

$$= 3.98$$

∴ SN_2 from table = 3.9

∴ $a_2 = 0.14$

∴ $m_2 = 0.80$

10 6
Finding SN_3 and D_3 .

$$D_3 = (SN_3 - SN_2) / a_3 m_3$$

$$= 6(4.4 - 3.98) / 0.10 \times 0.80$$

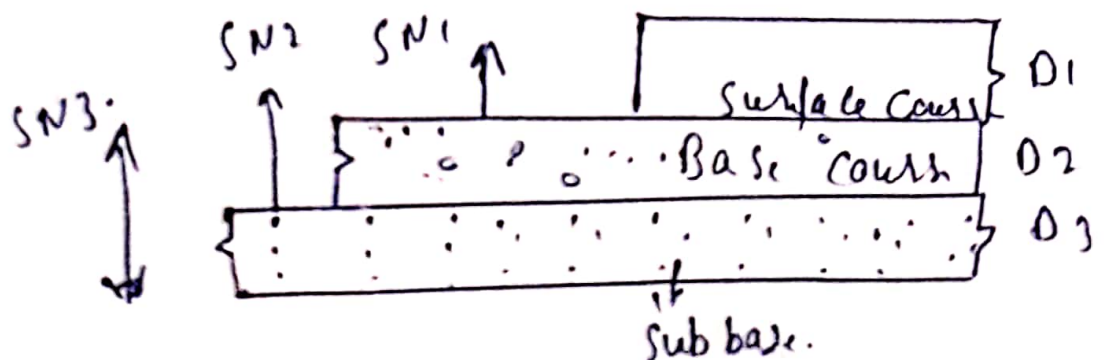
$$D_3 = 3.25''$$

We will use 8'' as sub base

$$SN_3 = 2.64 + 1.34 + 6'' \times 0.10 \times 0.80$$

$$SN_3 = 4.46 > 4.4 \text{ okay.}$$

Final Design \Rightarrow



11

Ashdo Design equation for ~~SP~~
SN.

$$\log_{10} W_{18} = ZRS_0 + 9.36 \log_{10} [SN+1] - 0.20$$

$$\log_{10} [\Delta PSI/4.1 - 1.5]$$

$$0.40 + [1094 / (SN+1)^{5.14}]$$

$$+ 2.32 \log_{10} M_r - 8.07$$

Q4 What are the different pavement distresses? Explain in detail?

⇒ Pavement Distresses :->

Distress is a condition of the pavement structure that reduces serviceability or leads to a reduction in service life.

→ Distresses could occur in a pavement due to.

- Unstable mixes
- Higher wheel loads than those considered in design.

→ Alligator (Fatigue) Cracking :->

→ Possible causes :->

- overloading
- inadequate structural design.
- poor construction.

→ Repair :-

- Crack sealing is in-effective
- Dig out and replace area of poor sub grade

Block cracking :

→ Problem: allow moisture infiltration.

→ Possible causes :->

- HMA shrinkage.
- Asphalt binder aging.
- Poor choice of asphalt binder in the mix design.

→ Repair:

- Low severity cracks ($< 1/2$ inch wide) crack seal prevent entry of moisture.

- High severity ($> 1/2$ inch wide and cracks with revealed edges) Remove and Replace the cracked pavement layer with an overlay.

Potholes :->

Small, bowl-shaped depressions in the pavement surface that penetrate all the way through the HMA layer down to the base course.

14
→ Problem :->

Roughness (serious vehicular damage can result from driving across potholes at higher speeds) moisture infiltration.

→ Possible causes :->

Generally potholes are the end result of fatigue cracking.

→ Repair :->

patching techniques.

→ Rutting :->

Surface depression in the wheel path, are particularly evident after a rain when they are filled with water

→ Possible causes :->

insufficient compaction of HMA layers during construction

subgrade rutting (e.g. excessively high asphalt content)

- Subgrade rutting: (e.g. as a result of inadequate pavement structure)

Repair:→

- Slight ruts ($< \frac{1}{3}$ " deep) can generally be left untreated, pavement with deeper ruts should be leveled and overlaid.

→ Bleeding:→

- problem: loss of skid resistance when wet.

→ Possible causes:→

Excessive asphalt binder in the HMA

- Low HMA air void content.

Polished Aggregate:

→ Possible causes:→

Repeated traffic application

this can occur quicker if the aggregate is susceptible to abrasion.

- Repair: Apply a skid-resistant seal BST, or non structural overlay.

⇒ Raveling :

⇒ loose debris on The Pavement which increases Pavement roughness and loss of skid resistance.

→ Possible causes :->

- Asphalt binder aging.
- Aggregate segregation.

if fine particles are missing from the aggregate matrix.

Repair :->

Fog seal / slurry seal or
Remove the damaged
Pavement and overlaid