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Section

B

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Subject

Hydraulic Engineering

Assignment

01, 02, 03

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Venturi flume :-

A Venturi flume is a critical flow. open flume with a constricted flow which causes a drop in the hydraulic grade line. Creating a critical depth.

→ It is used in flow measurement of very large flow rates usually given in millions of cubic units. A Venturi meter would normally measure in mm whereas a Venturi flume measure in meters.

Measurement of discharge with Venturi flumes requires two measurement.

One upstream and one at the throat.

Cross section if the flow passes in a

subcritical state through the flume. If the flumes are designed so as to pass the flow from subcritical to supercritical state while passing through the flume, a single measurement at the throat is sufficient for computation of discharge.

→ To ensure the occurrence of critical depth at the throat, the flumes are usually designed in such a way as to form a hydraulic jump on the downstream side of the structure. These flumes are called "standing wave flumes".

2- A 3m wide channel carries a total discharge of $12 \text{ m}^3/\text{sec}$ calculate.

* Critical depth.

* Minimum specific energy.

* Alternate depth, $E = 4\text{m}$

Given data

Wide of channel = 3m

Discharge, $Q = 12 \text{ m}^3/\text{sec}$

Solution

(a) Critical depth :-

Discharge per unit width

$$Q = \frac{Q}{b} = \frac{12}{3}$$

$$Q = 4 \text{ m}^2/\text{sec}$$

For Rectangular channel

$$A_c = \left(\frac{Q^2}{g}\right)^{1/3} = \left(\frac{42}{9.81}\right)^{1/3}$$

$$h_c = 1.18\text{m}$$

(b) Minimum Specific Energy $E_c = ?$

For Rectangular channel

$$E_c, \frac{3}{2} h_c = \frac{3}{2} \times 1.18$$

$$E_c = 1.77\text{m}$$

(c) The Alternate depth $E = 4\text{m}$

As $E > E_c$, there are two possible depths for a given specific energy

$$E = h + \frac{V^2}{2g} \quad \text{where} \quad V = \frac{Q}{A} = \frac{Q}{bh}$$

$$E = h + \frac{Q^2}{2gh^3}$$

$$4 = h + \frac{0.8155}{h^2}$$

for the subcritical solution
the first term, associated
with potential energy

$$h = \frac{4 - 0.8155}{h^2}$$

\Rightarrow Iteration (from $h=4$) gives $h=3.948\text{m}$
for subcritical (first shallows) solution.

The second term associated with kinetic
energy

dominates rearrange as:

$$\text{So, } h = \sqrt{\frac{0.8155}{4-h}}$$

Iteration (from $h=0$) gives $h=0.4814\text{m}$

So Alternate depth are 3.95m

and 0.4814m .

$$E = y + \frac{V^2}{2g}$$
$$= 0.1 + \frac{6^2}{2 \times 9.81}$$
$$= 1.935 \text{ m}$$

The alternate depth for $E = 1.935 \text{ m}$

Yields $y_{\text{alternate}} = 1.93 \text{ m}$

Q No 2

Given data

$$\text{Velocity} = V_1 = 2 \text{ m/s}$$

$$\text{depth} = y_1 = 3 \text{ m}$$

$$\text{Elevation } \Delta x = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{down step} = 15 \text{ cm} = 0.15 \text{ m}$$

Solution As we know that

$$E_1 = y_1 + \frac{U_1^2}{2g}$$

$$= 3 + \frac{2^2}{2 \times 9.81}$$

$$E_1 = 3.20 \text{ m}$$

Now $E_2 = E_1 - \Delta x$

$$= 3.2 - 0.6$$

$$E_2 = 2.60 \text{ m}$$

Also $E_2 = \frac{y_2 + \frac{U_2^2}{2g}}{y_2}$

$$2.60 = y_2 + \frac{6^2}{2 \times 9.81 y_2}$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_2 - y_1$$

$$\Delta y = 2.24 - 3$$

$$\Delta y = 0.76 \text{ m}$$

So water surface drop = 0.76 m

* for downward step of 15 cm or 0.15 m

we have

$$E_2 = E_1 - \Delta x = 3.20 - (-0.15)$$

$$E_2 = 3.55 \text{ m}$$

Now

$$y_2 = 3.17 \text{ m}$$

$$\Delta y = y_2 - y_1 = 3.17 - 3$$

$$\Delta y = 0.17 \text{ m}$$

So water surface rises 0.17 m

* The maximum step possible before
of losing upstream water surface level
is for

$$y = y_c$$

$$y_c = 3 \sqrt{\frac{q^2}{g}}$$

$$y_c = 3 \sqrt{\frac{6^2}{9.81}}$$

$$y_c = 1.54 \text{ m}$$

Assignment #03

Q No 1

Given data

$$y_1 = 3.6 \text{ m}, \quad y_2 = 0.9 \text{ m}, \quad b = 3.9 \text{ m}$$

Solution: As we know that

$$E_1 = E_2$$

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g} \rightarrow (1)$$

Also,

$$Q = A_1 V_1 = A_2 V_2$$

$$b y_1 V_1 = b y_2 V_2$$

$$b \cdot y_1 \cdot V_1 = b \cdot y_2 \cdot V_2$$

$$y_1 \cdot V_1 = y_2 \cdot V_2$$

$$V_2 = \frac{y_1}{y_2} \times V_1$$

$$V_2 = \frac{3.6}{0.9} \times V_1$$

$$V_2 = 4V_1 \rightarrow (2)$$

Putting in eq (1)

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g}$$

$$3.6 + \frac{V_1^2}{2g} = 0.9 + \left(\frac{4V_1}{2g}\right)^2$$

$$3.6 + \frac{V_1^2}{2g} = 0.9 + \frac{16V_1^2}{2g}$$

$$\frac{V_1^2}{2g} - \frac{16V_1^2}{2g} = 0.9 - 3.6$$

$$\frac{V_1^2 - 16V_1^2}{2g} = -2.7$$

$$\frac{-15V_1^2}{2g} = -2.7$$

$$V_1^2 = \sqrt{\frac{2.7 \times 2(9.81)}{15}}$$

$$V_1 = 1.879 \text{ m/sec}$$

Put in eq

$$V_2 = 4V_1$$

$$V_2 = 4(1.879)$$

$$V_2 = 7.516 \text{ m/sec}$$

As $Q_1 = A_1 V_1 = b y_1 \cdot V_1$

$$3.9 \times 3.6 \times 1.879$$

$$Q_1 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 V_2 = b y_2 \cdot V_2$$

$$= 3.9 \times 0.9 \times 7.516$$

$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

(ii) Froude number \rightarrow At upstream side

$$Fr_1 = \frac{V_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}} = 0.31$$

Subcritical flow

b) Froude Number \rightarrow at downstream stream

$$Fr_2 = \frac{V_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}} = 2.52$$

(Super critical flow)