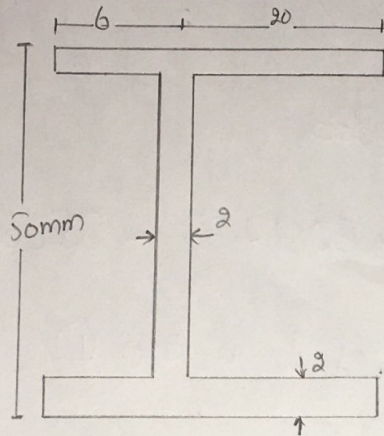


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## QUESTION No.1(a)

Determine the location of the shear center--  
 ----- based on the centerline dimensions.



## GIVEN DATA:

Height,  $h = 50 \text{ mm}$

Breadth,  $b = 26 \text{ mm}$

## REQUIRED DATA:

We have to find the location of shear center.

## SOLUTIONS:

As we know this

$$e = \frac{t_f h^2 b^2}{4I} \quad \text{--- ①}$$

First we have to find "I"



$$I = 2 \left( \frac{bh^3}{12} + Ay^2 \right) + \left( \frac{bh^3}{12} + Ay^2 \right)$$

$$\Rightarrow I = 2 \left( \frac{26(2)^3}{12} + (26 \times 2)(25)^2 \right) + \left[ \frac{2(50)^3}{12} + 0 \right]$$

$$\Rightarrow I = 2(32517.33) + 20833$$

$$\Rightarrow I = 53350.66 \text{ mm}^4 + 32517.33 = 85867.99 \text{ mm}^4$$

Now put the value of I in eqn ①

$$\Rightarrow e = \frac{2(50)^2(26)^2}{4(85867.99)}$$

$$\Rightarrow e = 9.8406 \text{ mm}$$

RESULT:

Hence  $e = 9.8406 \text{ mm}$

### QUESTION No. 1(b)

Determine the thickness of the wall -----

----- weight of water is  $62.4 \text{ lb/ft}^3$

GIVEN DATA:

$$h = 26 \text{ ft} = 26 \times 12 = 312 \text{ inch}$$

Tangential

$$\text{Stress} = 6000 \text{ psi}$$

Specific

$$\text{weight} = 62.4 \text{ lb/ft}^3 = 62.4 \times \frac{1}{(12)^3} = 0.036 \text{ lb/inch}^3$$



Diameter,  $D = 22 \text{ ft} = 22 \times 12 = 264 \text{ inches.}$

REQUIRED DATA:

Thickness,  $t = ?$

SOLUTION:

As

$$\sigma_t = \frac{PD}{2t} \quad \text{--- (1)}$$

Where Pressure exerted by water is

$$P = \gamma h$$

$$\Rightarrow \sigma_t = \frac{\gamma h D}{2t}$$

$$\Rightarrow t = \frac{\gamma h D}{2\sigma_t}$$

$$\Rightarrow t = \frac{(0.036)(312)(264)}{6000 \times 2}$$

$$\Rightarrow t = 0.247''$$

$$\Rightarrow t = \boxed{0.0205 \text{ ft}}$$

RESULT:

Hence thickness  $t = 0.0205 \text{ ft.}$



## QUESTION No. 2(a)

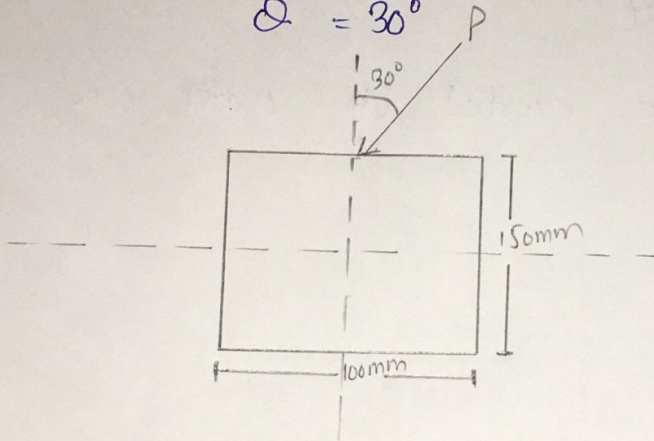
The 100 by 150 mm wooden beam  
 Neglect the weight of the beam.

GIVEN DATA:

$$\text{Load, } P = 4 \text{ kN}$$

$$\text{Length, } L = 3 \text{ m}$$

$$\alpha = 30^\circ$$



REQUIRED DATA:

Bending stress,  $\sigma = ?$

Location of neutral axis,  $\alpha = ?$

Solution:

Now as we know that

$$\sigma = \sigma_x + \sigma_y \quad \text{--- (1)}$$

$$\sigma_x = \frac{Mx y}{I_x} \quad \text{--- (2)}$$

Now first we find the  $Mx$



$$M_x = P \cos \theta$$

$$\Rightarrow M_x = 4 \times 10^3 \cos(30^\circ)$$

$$\Rightarrow \boxed{M_x = 3.46 \times 10^3}$$

$$\text{Now } I_x = \frac{bh^3}{12}$$

$$\Rightarrow I_x = \frac{0.1(0.15)^3}{12}$$

$$\Rightarrow \boxed{I_x = 2.8125 \times 10^{-5} \text{ m}^4}$$

Now put values in equ ②

$$\sigma_x = \frac{3.46 \times 0.075 \times 10^3}{2.8125 \times 10^{-5}}$$

$$\Rightarrow \boxed{\sigma_x = 9.237 \text{ MNm}^{-2}}$$

Now

$$\sigma_y = \frac{M_y x}{I_y} \quad \text{--- ③}$$

$$M_y = P \sin \theta$$

$$\Rightarrow M_y = 4 \times 10^3 \sin 30^\circ$$

$$\Rightarrow \boxed{M_y = 2000}$$

$$\text{Now } I_y = \frac{hb^3}{12}$$



$$\Rightarrow I_y = \frac{0.15 (0.1)^3}{12}$$

$$\Rightarrow I_y = 1.25 \times 10^{-5} \text{ m}^4$$

Now putting values in equ ③

$$\sigma_y = \frac{2000 \times 0.05}{1.25 \times 10^{-5}}$$

$$\Rightarrow \sigma_y = 8 \times 10^6$$

$$\Rightarrow \sigma_y = 8 \text{ MNm}^{-2}$$

Now putting values in equ ①

$$\sigma = -\sigma_x + \sigma_y$$

$$\Rightarrow \sigma = -9.237 + 8$$

$$\Rightarrow \sigma = -1.237 \text{ MNm}^{-2} \quad (\text{Compression})$$

Now for Neutral axis of unsymmetrical Bending.

$$\tan \alpha = \frac{I_x}{I_y} \cdot \frac{M_x}{M_y}$$

$$\Rightarrow \tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} \times \frac{3.46 \times 10^3}{2 \times 10^3}$$



$$\Rightarrow \alpha = \tan^{-1}(3.8925)$$

$$\Rightarrow \boxed{\alpha = 75.59^\circ}$$

RESULT:

Hence  $\sigma = -1.237$  (compression)

$$\alpha = 75.59^\circ$$

### QUESTION No. 2(b)

GIVEN DATA:

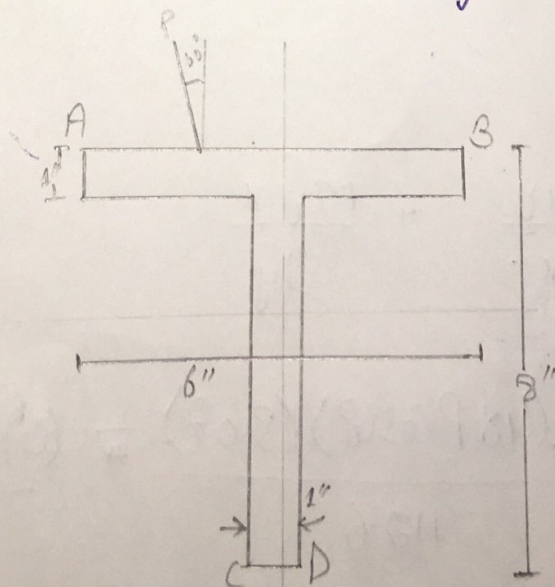
$$\text{length, } L = 16 \text{ ft.}$$

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7$$

$$\text{Compressive strength} = 12000 \text{ psi}$$

$$\text{Tensile strength} = 5000 \text{ psi}$$





REQUIRED DATA:

Maximum load,  $P = ?$

SOLUTION:

As we know that

$$M = \frac{PL}{4}$$

$$\Rightarrow M_x = \frac{(P \cos 60^\circ)(16 \times 12)}{4}$$

$$M_y = \frac{(P \sin 60^\circ)(16 \times 12)}{4}$$

Now we have to find stresses at A, B, C and D.

At Point A:

$$\sigma_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$\Rightarrow \sigma_A = -\frac{(48 P \cos 60^\circ)(3.07)}{112.6} + \frac{(4 P \sin 60^\circ)(3.07)}{18.7}$$

$$\Rightarrow \sigma_A = -0.654P - 6.6P$$

$$\Rightarrow \sigma_A = -7.2P \text{ (compression)}$$



$\epsilon$  Compression  $\leq 12000 \text{ Psi}$

$$12000 = -7.25P$$

$$\Rightarrow \boxed{P = 1655.11 \text{ lb}}$$

At Point B:

$$\sigma_B = \frac{-(48P \cos 60^\circ)(3.07)}{112.6} + \frac{(48P \sin 60^\circ)(3.07)}{18.7}$$

$$\Rightarrow \sigma_B = -0.654P + 6.6P$$

$$\Rightarrow \sigma_B = 6.01P \text{ (Tension)}$$

$\epsilon$  Tension  $\leq 5000 \text{ Psi}$

$$5000 = 6.01P$$

$$\Rightarrow \boxed{P = 831.94 \text{ lb}}$$

We have consider minimum value of

$P$

$$\text{so } \boxed{P = 831.94 \text{ lb}}$$

At Point C:

$$\sigma_C = + \frac{(48P \cos 60^\circ)(3.07)}{112.6} + \frac{(48P \sin 60^\circ)(3.07)}{18.7}$$



$$\Rightarrow \sigma_c = +0.654P + 6.6P$$

$$\Rightarrow \sigma_c = 7.25P \text{ (Tension)}$$

$$\& \text{ Tension} \leq 5000 \text{ Psi}$$

$$5000 = 7.25P$$

$$\Rightarrow \boxed{P = 689.65 \text{ lb}}$$

At Point D:

$$\sigma_D = + \frac{(48P \cos 60^\circ)(3.07)}{112.6} - \frac{(48P \sin 60^\circ)(3)}{18.7}$$

$$\Rightarrow \sigma_D = +0.654P - 6.6P$$

$$\Rightarrow \sigma_D = -5.946P \text{ (Compression)}$$

$$\& \text{ Compression} \leq 12000 \text{ Psi}$$

$$\Rightarrow 12000 = -5.946P$$

$$\Rightarrow \boxed{P = 2018.16 \text{ lb}}$$

So we have consider Minimum value of P so;

$$\boxed{P = 689.65 \text{ lb}}$$



## QUESTION No.3

GIVEN DATA:

$$\text{length, } L = 10 \text{ ft}$$

$$\text{Breath, } b = 0.75''$$

$$\text{height, } h = 2''$$

$$\text{Factor of safety} = 2$$

$$E = 10.3 \times 10^6 \text{ Psi}$$

REQUIRED DATA:

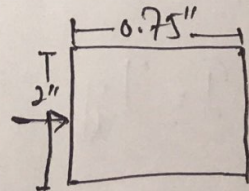
$$\text{Safe load, } P_{\text{safe}} = ?$$

SOLUTION:

CASE I:

Strut column act as hinged column about an axis perpendicular to the 2" dimension then

$$I = I_x = \frac{(0.75)(2)^3}{12} = 0.5 \text{ in}^4$$



$l_e = L$  (For hinged ended column)

$$P_{cr} = n^2 \frac{EI\pi^2}{(l_e)^2}$$



$$\Rightarrow P_{cr} = (1)^2 \frac{(10.3 \times 10^6) (0.5) (3.14)^2}{(10 \times 12)^2}$$

$$\Rightarrow \boxed{P_{cr} = 3526.71 \text{ lb}}$$

For  $P_{safe}$ :

$$P_{safe} = \frac{P_{cr}}{\text{Factor of Safety}}$$

$$\Rightarrow P_{safe} = \frac{3526.7}{2}$$

$$\Rightarrow \boxed{P_{safe} = 1763.35 \text{ lb}}$$

CASE II:

Column act as fixed end about axis parallel to z in i.e y axis.

$$I = I_y = \frac{(2)(0.75)^3}{12}$$

$$\Rightarrow \boxed{I_y = 0.07 \text{ in}^4}$$

For Fixed ended column

$$L_e = L/2$$



$$P_{cr} = \frac{n^2 EI \pi^2}{L^2}$$

$$\Rightarrow P_{cr} = \frac{(1)^2 (10.3 \times 10^6) (0.07) (3.14)^2}{(120/2)^2}$$

$$\Rightarrow P_{cr} = 1974.65 \text{ lb}$$

For  $P_{safe}$ :

$$P_{safe} = \frac{P_{cr}}{\text{Factor of safety}}$$

$$\Rightarrow P_{safe} = \frac{1974.65}{2}$$

$$\Rightarrow P_{safe} = 987.32 \text{ lb}$$

In both cases we take smaller value of  $P_{safe}$ :

$$P_{safe} = 987.32 < 1763.07$$