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Section

A

Semester

6th

Subject

irrigation engineering

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Qa 1 Name Shohab malook
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Section A

Q No 1

Explain Anti water logging measures?

Ans..

Control of canal seepage -
Canal seepage is major source of water losses and logging in irrigation areas and it can be controlled by lining of canal with impervious material like clay concrete to control seepage. Convert water system from open canal to piped system.

Reducing intensity of irrigation:

The most important aspect to avoid water logging is to provide the water to thus small portion of land where necessary.

Applying where necessary only the required amount the water so that all water applied is used by plant use efficient irrigation method i.e. drip irrigation.

Rotation of crop:

It means that we should plant crop in such way that it prevent the land from water logging.

Crop which used large amount of water should be followed by those plants which used less water or no water.

2 No 16

Differentiate between Saline and alkaline soils.

Ans:-

Saline Soil:

By the principle of osmosis, pure water from roots flow out from in plant are due to lack of water such soil is unproductive and called Saline Soil. Saline Soil appearance is a black crusty core over the surface of earth.

Alkaline Soil:

If the salt efflorescence continues for a longer period a base exchange reaction with clay take place thus solidifying the clay making it impermeable illaerated and highly unproductive such soil are called alkaline soils.

It is white in appearance as white patches appear over earth surface.

~~Q No~~
Q No 10

How do you reclaim salt affected lands?

Ans:

Following are the major aspects to reclaim salt affected lands.

The practice of crop reversal is necessary to reduce the establishment of salt tolerant crops.

An efficient drainage (surface & subsurface) must be provided to lower the water table in saline soils.

High salt resistant crops like rice are grown leached land for 1 or 2 seasons.

Land should be flooded with water so that alkaline soil will dissolve water.

Q No 2a Explain the procedure of desilting of an irrigation canal by Kennedy's theory.

Ans: Kennedy's theory:

-> R.L Kennedy studied straight reaches of upper Bari Doab canal which are stable 30 years.

-> $V_0 = CD^n$

Where V_0 is critical velocity (non-silting or non scouring)

C is constant depend upon quantity of silt.

Sediment is kept in suspension solely by the vertical component of eddies.

-> Weight of sediment vertically act downwards.

-> vertical component of eddies acts upwards

-> Result the sediment is in suspension

-> $V_0 = 0.84 D^{0.64}$ FPS system

-> $V_0 = 0.546 D^{0.64}$ MKS system, D is depth.

-> $V_0 = 0.546 m D^{0.64}$ where $M = V/V_0 =$ critical velocity ratio (C.V.R) depend upon silt grade.

-> $V =$ critical velocity for all size of sediment V_0 is V_{cr} for upper Bari Doab only.

Kennedy procedure for canal design.

Step 1: Assume the trial value of D and put in eq (1) and determine

$$V_0 = 0.546 m D^{0.64}$$

Step 2: in eq 1: $Q = AV$

$$A = Q/V$$

$$A = BD + D^2/2$$

$$P = B + D s^{1/2}$$

for assumed D determine B

Find $R = A/P$.

Step 3: Substitute the value of R in eq (2) (Kutters and Chazys eq) to obtain v which will be the actual velocity for assumed dimensions.

Step 4: if the velocity worked out from eq (2) agrees with that of obtained with the eq (3) (Kennedys eq) then the assumed depth is correct otherwise repeat the procedure with change value of D .

Q2b1

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Q2b1: Design an irrigation channel by Kennedy's theory to carry a discharge of 30 cumec with $C_u(m)$ of 1 and N as 0.0225 and bed slope of 1 in 5000. Assume the depth (D) as 2.3m.

Given Data.

$$\text{Discharge (Q)} = 30 \text{ m}^3/\text{sec}$$

$$C_u, (m) = 1$$

$$\text{Bed Slope} = 1 \text{ in } 5000$$

$$N = 0.0225$$

$$\text{Depth} = 2.3 \text{ m}$$

Solution.

Finding velocity.

$$V_k = 0.546 \text{ m } D^{0.64}$$

$$= 0.546 (1) (2.3)^{0.64} = \boxed{V_k 0.93}$$

Now, calculating Area of canal

$$Q = AV \Rightarrow A = Q/V$$

$$A = 30/0.930 \quad \boxed{\text{Area} = 32.25 \text{ m}^2}$$

Now we have to calculate B by using formula.

$$A = BD + \frac{D^2}{2} = BD + 0.5 D^2$$

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By putting values

$$A = BD + 0.5 D^2$$

$$32.25 + B(2.3) + 0.5(2.3)^2$$

$$32.25 - 2.645 = 2.3(B)$$

$$29.605 = 2.3(B)$$

$$B = 12.87 \text{ m}$$

Now we have to calculate wetted perimeter

$$P = B + \sqrt{5} D$$

$$= 2.87 + \sqrt{5}(2.3) = P = 18.01 \text{ m}$$

we have to calculate Hydraulic Radius

$$R = A/P$$

$$= 32.25/18.01 = R = 1.79 \text{ m}$$

Now calculating mean velocity from Chezy equation.

$$V_c = C(RS)^{1/2}$$

$$C = \frac{1/n + 23 + 0.00155}{1 + (23 + 0.00155) n/\sqrt{R}}$$

$$= \frac{1/0.0225 + (23 + 0.00155)}{1 + (23 + 0.00155) \frac{1}{\sqrt{5000}}}$$

$$C = 49.56 \frac{1}{\sqrt{5000}} \times \left(\frac{0.0225}{\sqrt{1.79}} \right)$$

$$V_c = 49.56 (1.79 (1/\sqrt{5000}))^{1/2}$$

$$V_c = 0.93 \text{ m}$$

Qno 3a Differentiate between initial regime and final regime in accordance to Lacey's theory.

Ans: initial Regime:

When only bed slope of channel change but the cross section remains same than also no silting or scouring take place. But this is rare.

- > bed slope of a channel varies
- > cross-section or wetted perimeter remains unaffected.

Final Regime:

if all the parameters (perimeter depth and slope) have equally free to vary and adjust according to discharge and silt grades then the channel is said to have final regime.

all the variables such as perimeter depth slope etc are equally free to vary and achieve permanent stability called final Regime.

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Q.No 3b

Design a regime channel by Lacey's theory for discharge of 30 cumecs and mean diameter of silt particle of 0.56mm.

Given Data

$$Q = 30 \text{ m}^3/\text{sec}$$

$$m = 0.56 \text{ mm}$$

Solution.

$$\text{Silt Factor} = 1.76 \times m^{0.5}$$

$$f = 1.76 \times (0.56)^{0.5}$$

$$f = 1.3$$

$$V_m = \left[\frac{Q f^2}{140} \right]^{1/6}$$

$$= \left(\frac{30 \times (1.3)^2}{140} \right)^{1/6}$$

$$V_m = 0.844$$

$$Q = AV = A \times V = 30 / 0.844$$

$$A = 35.54$$

$$P = 4.75 \sqrt{Q}$$

$$P = 4.75 \sqrt{30}$$

$$P = 26.01$$

$$R = \frac{5}{2} \times \frac{V^2}{f} = \frac{5}{2} \times \frac{(0.844)^2}{1.3}$$

$$R = 1.36$$

(3b) 2

$$A = BD + \frac{D^2}{2}$$

$$35 \cdot 54 = BD + \frac{D^2}{2} \quad \text{--- (1)}$$

$$P = B + D\sqrt{5}$$

$$26.01 = B + 2.236D$$

$$B = 26.01 - 2.236D \quad \text{--- (2)}$$

put eq (2) in eq (1)

$$35 \cdot 54 = (26.01 - 2.236D)D + \frac{D^2}{2}$$

$$35 \cdot 54 = 26.01D - 2.236D^2 + \frac{D^2}{2}$$

$$35 \cdot 54 = 26.01D - 2.236D^2 + 0.5D^2$$

$$35 \cdot 54 = 26.01D - 1.736D^2$$

$$-\frac{1.736D^2}{a} + \frac{26.01D}{b} - \frac{35 \cdot 54}{c} = 0$$

$$a = -1.736 \quad b = 26.01 \quad c = -35 \cdot 54$$

By Quadratic eq

$$D = \frac{-(-26.01) \pm \sqrt{(-26.01)^2 - 4(-1.736)(-35 \cdot 54)}}{2(-1.736)}$$

$$D = 1.52$$

put in eq (2)

$$B = 26.01 - 2.236(1.52)$$

$$B = 22.611$$

$$S = f(5/3)$$

$$\frac{33400^{1/6}}$$

$$S = \frac{1.3}{3340(30)^{1/6}}$$

$$S = 0.00026$$

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Q No 40: Explain the component of headworks with neat diagram.

Ans weir or barrage

Weir:- Normally the water level of any perennial river is such that it cannot be diverted to the irrigation canal the bed level of the canal may be higher than the existing water level of the river. adjustable shutters are provided on the crest to raise the water level to some required height.

Barrage:- when the water level on the up stream side of the weir is required to be raised to different level at different time barrage is constructed. Barrage is an arrangement of adjustable gates or shutter at different times over the weir.

under sluices:

also known as Scouring Sluices the under Sluices are the opening provided at the base of the weir or barrage these opening are provided with adjustable gates normally the gates are kept closed the suspended silt goes on depositing in front of the canal head regulator when the silt depositing becomes appreciable the gates are opened and the deposited silt is loosened with an agitator mounting on boat The gates are then closed but at the period of flood the gates are kept opened.

Divid wall:

The divid wall is a long wall constructed at right angles in the weir or barrage it may be constructed with stone masonry or cement concrete on the upstream side the wall is extended just to cover the canal head regulator and on the downstream side it is extended up to the launching apron.

The functions of the divide wall are flowing to form a still water pocket in front of the canal head so that the suspended silt can be settled down which then later be cleaned through the ~~scour~~ scouring sluices from time to time. It resists the overturning effect on the weir or barrage caused by the pressure of the impounding water.

Fish ladder:

The fish ladder is provided just by the side of the divide wall for the free movement of fishes. Rivers are important source of fishes. The tendency of fish is to move from downstream to upstream in monsoons. This movement is essential for their survival. Due to construction of weir or barrage, this movement gets obstructed and is detrimental to the fishes.

Canal head regulator:

A structure which is constructed at the head of the canal to regulate flow water is known as canal head regulator. It consists of a number of piers which divide the total width of the canal in number of spans which are known as bays.

The piers consist of number tiers on which the adjustable gates are placed.

Further again some piers are constructed on the down stream side of the canal head to support the roadway.

Function of Canal Head Regulator:

It regulates the supply of water entering the canal. It controls the entry of silt in the canal. It prevents the river's flood from entering the canal.

Silt regulation works:

The entry of silt into a canal which takes off from a head work can be reduced by constructing certain special work called silt control work.

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Silt excluders:-

Silt excluder are those work which are constructed on the bed of the river upstream of the head regulator. The cleared water enters the head the silt excluded in this type of work the silt is therefore removed from the water before it enters the canal.

Silt Ejectors:-

Silt ejectors also called Silt extractor are the devices which extract the silt from the canal water after the silted water has travelled a certain distance in the off take canal those work are therefore constructed on the the bed of the canal and little distance downstream from the head regulator.

River training works:

River training works are required near the weir site in order to ensure a smooth and an axial flow of water and thus to prevent the river from outflanking the works due to a change in its course the river training work required on a canal headwork are.

Guide bank

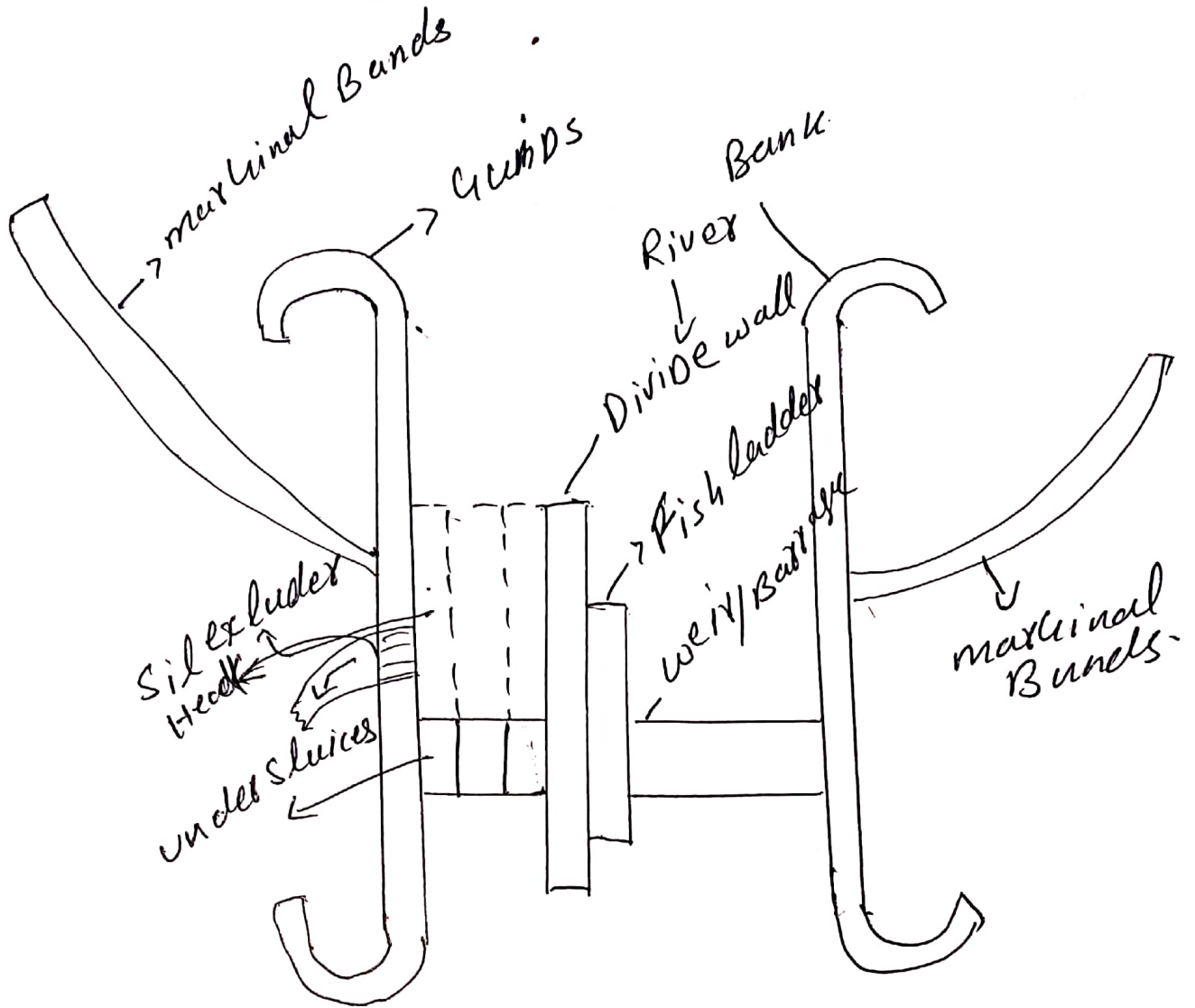
When a barrage is constructed across a river which flow through the alluvial soil the guide banks must be constructed on both the approaches to protect the structure from erosion. Guide bank serves the following purposes. it controls the velocity of flow near the structure.

marginal Bunds.

The marginal Bunds are earthen embankment which are constructed parallel to the river bank on one or both the banks according to the condition the top width is generally 3" to 4 m. the side slope on the river side is generally 1.5:1 and that on the country side is 2:1.

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NOYB

water are the function of Head regulators?

Ans:

Function of Head regulators:

- > it regulates the supply of water entering the canal
- > it control the entry of silt in the canal
- > it prevent the river floods from entering the canal

The functions of canal head regulator are to regulate the supplies in the canal to indicate the discharge passed into the canal from design discharge formula and observed head of water on the crest to control the silt entry in the canal.