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Summer  
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Subject: - Advance  
Fluid  
Mechanics

# Q # 1 (Part-A)

## DRAAG :-

In fluid dynamics drag is known as air resistance, a type of friction or fluid friction. Resistance or type of friction or fluid friction is a force acting opposite to the relative motion of any object moving with respect to a surrounding fluid. This can exist between two fluid layers (or surfaces) or a fluid and a solid surface.

It has two components

- 1. Pressure Drag
- 2. Friction Drag.

### 1. (PRESSURE DRAG) (FP) :-

It is equal to the integration of component in direction of motion of all pressure forces exerted on surface of body.

~~$$F_p = C_p \rho \frac{v^2}{2} A$$~~

where "Cp" depends on shape.

$$F_p = C_p \rho \frac{v^2}{2} A$$

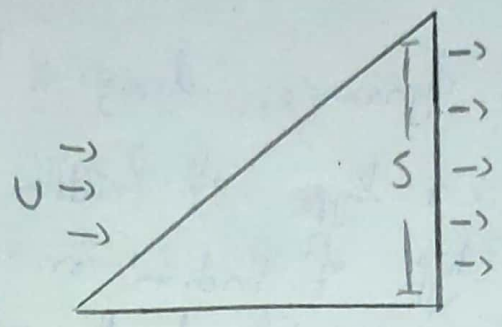
### 2) FRICTION DRAG :-

It is equal to integration of components of shear stress along the surface of the body in direction of motion

$$F_f = C_f \rho \frac{v^2}{2} BC$$

Cf → depends on viscosity.

# FRICTION DRAG OF BOUNDARY



$$\bar{\tau}_0 = \mu \frac{U \int f(\eta) d\eta}{\delta \int f(\eta) d\eta}$$

$$\bar{\tau}_0 = \frac{\mu U B}{\delta}$$

As we know have  $\tau_0 = \int U^2 \times d\delta \frac{d\delta}{dx}$

Compare both

$$\frac{\int U^2 d\delta}{dx} = \frac{\mu U B}{\delta}$$

$$\delta d\delta = \frac{\mu B}{\int U^2} dx$$

Integrating on both sides.

$$\frac{\delta^2}{2} = \frac{\mu B}{\int U^2} x + C$$

∴ C = 0

$$\delta = \sqrt{\frac{2B}{d}} \cdot \sqrt{\frac{\mu u}{\rho U}}$$

$$\therefore R_{\delta} = \frac{U \delta \rho}{\mu}$$

$$B = 1.63, \quad d = 0.135$$

$$\delta = \frac{4.91}{\sqrt{R_n}} x \rightarrow \textcircled{B}$$

where  $(R_n)$  is local Reynold Number

we have

$$\tau_0 = \frac{\mu U B}{\delta}$$

$$F_n = \int B U^2 \delta dx$$

where  $d$  is a function of Boundary Layer velocity distribution

Now to find Shear Stress

$$\tau = \frac{F_n}{A} = \frac{d F_n}{B dx} = \frac{d F_n}{B dx}$$

$$\tau_0 = \frac{\int B U^2 d \delta}{B dx} = \int U^2 \alpha \frac{d \delta}{dx}$$

$$\tau_0 = \int U^2 \alpha \frac{d \delta}{dx} \quad - \text{General Equation}$$



# LAMINAR BOUNDARY LAYER

$$\frac{u}{U} = f\left(\frac{y}{\delta}\right) \quad \text{--- (1)}$$

$$\frac{y}{\delta} = \eta \quad \Rightarrow y = \delta \eta$$

$$dy = \delta d\eta \quad \text{--- (2)}$$

$$\frac{u}{U} = \cancel{f\left(\frac{y}{\delta}\right)} = f(\eta)$$

$$du = U df(\eta) \quad \text{--- (3)}$$

for laminar flow

$$\tau_0 = \mu \frac{du}{dy} \quad \text{--- (4)}$$

$$\tau_0 = \frac{\mu U df(\eta)}{\delta d(\eta)}$$

$$\tau_0 = \frac{\mu U \beta}{\delta} \quad \text{--- (5)}$$

As we have  $\tau_0 = \int_0^{\delta} \mu \frac{du}{dy} d\delta$

compare both

$$\int_0^{\delta} \mu \frac{du}{dy} d\delta = \frac{\mu U \beta}{\delta}$$

$$\delta d\delta = \frac{\mu \beta}{\rho U}$$

Integrating on both sides

$$\frac{\delta^2}{2} = \frac{\mu B}{\rho U} x + C \quad \therefore C = 0$$

$$\delta = \sqrt{\frac{2B}{\rho}} \cdot \sqrt{\frac{\mu x}{U}} \quad \therefore R_n = \frac{U x \rho}{\mu}$$

$$\delta = \frac{4.91x}{\sqrt{R_n}} \quad \text{--- (5)}$$

where  $(R_n)$  is Local Reynold Number  
so we have

$$z_0 = \frac{\mu U B}{\rho}$$

Put 6 in Equation 5

$$z_0 = 0.332 \frac{\mu U}{\rho} \sqrt{R_n}$$

Now

$$F_f = B \int_0^x z_0 dx$$

$$\text{where } z_0 = 0.332 \frac{\mu U}{\rho} \sqrt{R_n}$$

$$\therefore R_n = \frac{\rho U x}{\mu}$$

then putting values we have

$$F_f = 0.664 \sqrt{\rho \mu U^3 x}$$

we have

$$F_f = C_f \int \frac{U^2}{2} B dx$$

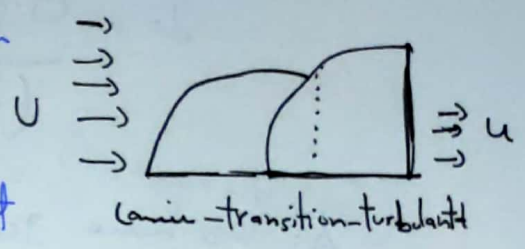
thus Equation both side.

$$cf = 1.328 \sqrt{\frac{\mu}{\sigma_{CU}}} = \frac{1.328}{\sqrt{Rn}}$$

for Laminar  $R \geq 500,000$

# TURBULANT BOUNDARY LAYER:-

This fig shows the velocity distribution of boundary layer which is steeper near walls & flatter throughout remainder of layer.



The Shear Stress is greater in turbulent than in laminar thus

$$\tau_0 = \int f \frac{v^2}{8}$$

where  $v$  is the average velocity to obtain relation between average and mass we have.

$$\frac{v}{u_{max}} = \frac{1}{1.33 \sqrt{f}} \quad \therefore f = 0.028$$

$$\frac{v}{u_{max}} = \frac{1}{1.233 \sqrt{0.028}}$$

$$u = 1.235 v$$

$$v = \frac{u}{1.235}$$

$$f = 0.316$$

$$(R_n)^{1/4}$$

$$\therefore R_n = \left(\frac{D u}{\nu}\right)$$

$$D = 2\delta$$

$$\tau_0 = f \rho \frac{u^2}{8}$$

$$\tau_0 = 0.316$$

$$\left(\frac{D}{\nu}\right) \left(\frac{u}{1.235}\right)^{1/4}$$

$$f/8 \left(\frac{u}{1.235}\right)^2$$



$$Z_0 = \frac{0.023 f U^2}{\left(\frac{2d}{V}\right)^{1/4}} \quad - (1)$$

As we have general

$$Z_0 = f U^2 \propto \frac{d \delta}{du} \quad - (2)$$

Equation 1 & 2

$$u=0, \delta=0$$

$$S = \left(\frac{0.0287}{\alpha}\right)^{4/5} \left(\frac{V}{u\alpha}\right)^{1/5} \cdot \alpha$$

$$d = 0.0972$$

$$S = \frac{0.377}{(Re)^{1/5}} \cdot \alpha \quad - (3)$$

$$Z_0 = 0.0587 \int \frac{U^2}{2} \left(\frac{V}{u\alpha}\right)^{1/5}$$

$$F_d = B \int_0^L Z_0 dx$$

$$F_d = 0.0735 \int \frac{U^2}{2} \left(\frac{V}{u\alpha}\right)^{1/5} \cdot B dx$$

$$F_d = C_f \int \frac{V^2}{2} \cdot B dx$$

Equating on both sides

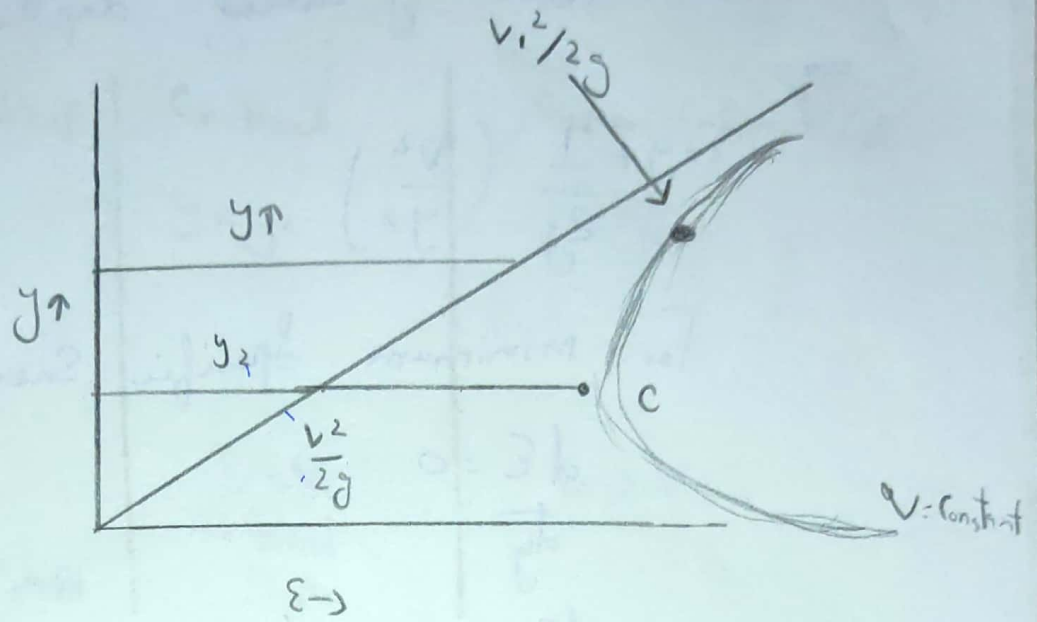
$$Cf = \frac{0.0735}{(R)^{1/5}}$$

$$(500000 < R < 10^7)$$

For  $e R > 10^7$

$$Cf = \frac{0.555}{(\log R)^{2.52}}$$

Q2 Part (b)



This is Specific Energy equation:-

For particular  $q$ , there will be two kind of possible values of  $y$  for given  $E$ . The equation is cubic with three roots with third being negative giving no values. Thus two Alternative Depths represents two totally different flow regimes - slow and deep or upper position & Fast & shallow & deep and upper position.

Point represent dividing point between two regimes of flow.

Thus for given " $q$ " value of  $E$  is minimum & flow at this point is critical flow. Depth of flow at at this point is for critical depth  $L/C$

And velocity at this point is Critical Velocity.

Thus Relation of Critical depth can be found as

$$E = y + \frac{1}{2g} \left( \frac{v^2}{y^2} \right)$$

For minimum Specific Energy

$$\frac{dE}{dy} = 0$$

$$\frac{dE}{dy} = 1 - \frac{2}{2g} \left( \frac{v^2}{y^3} \right)$$

$$\frac{dE}{dy} = 1 - \frac{v^2}{gy^3}$$

$$1 = \frac{v^2}{gy^3} = v^2 = gy^3$$

$$y_c = \left( \frac{v^2}{g} \right)^{1/3}$$

$$\text{As } v = vy, \quad v_c^2 = gy^3$$

$$\text{or } v_c = \sqrt{gy_c}$$

$$y_c = \frac{v_c^2}{g}$$

$$\text{Now } \frac{y_c}{2} = \frac{v_c^2}{2g}$$

$$E_{\min} = y_c + \frac{v_c^2}{2g} = y_c + \frac{y_c}{2}$$



$\frac{3}{2} y_c$  or  $y_{cv} = \frac{2}{3} \text{ constant}$

	Sub-Critical	Critical	Super-Critical
Depth of flow	$y > y_c$	$y = y_c$	$y < y_c$
velocity	$v < v_c$	$v = v_c$	$v > v_c$
Slope	mild slope $S_0 < S_c$	Critical slope	

# Q #2

Given:-  
Depth of Rectangular Channel (d) =:

$$\text{Flow rate (Q)} = 3.5 \text{ m}^3/\text{Sec}$$

$$\text{Slope of Bed (S}_0\text{)} = 0.0008$$

$$n = 0.0219$$

$$\begin{aligned} \text{width of Bed} &= 7.386 \text{ mm} \\ &= 7.386 \end{aligned}$$

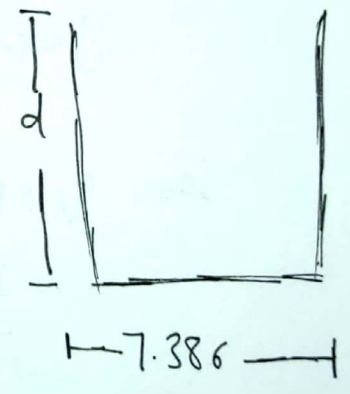
Critical Depth:

flow Sub Critical or Super Critical

Sol:-

$$\begin{aligned} \text{Area} &= 7.386 \times d \\ &= 7.386d \end{aligned}$$

$$\begin{aligned} \text{Perimeter} &= d + 7.386 + d \\ &= 7.386 + 2d \end{aligned}$$



$$\begin{aligned} \text{Hydraulic Radius (Rh)} &= A/P \\ &= \frac{7.386d}{7.386 + 2d} \end{aligned}$$

By using Manning Equation.

$$Q = \frac{1}{n} ARn^{2/3} \cdot S_0^{1/2}$$

Putting values

$$3.5 = \frac{1}{0.0219} \times \cancel{7.386} d \times \left( \frac{7.386}{2d + 7.386} \right)^{2/3} \times (0.0008)^{1/2}$$

$$d = 0.58 \text{ m}$$

$$\text{Area} = 7.386 (0.58) = 4.28 \text{ m}^2$$

$$\text{Perimeter} = 7.386 + 2(0.58) = 8.54 \text{ m}$$

$$\text{Hydraulic Radius (Rh)} = \frac{4.28}{8.54} = 0.511 \text{ m}$$

Finding Critical Depth:

$$y_{cr} = \left( \frac{Q^2}{g} \right)^{1/3}$$

$$\begin{aligned} \text{As } Q &= Q/B \\ &= 3.5 \\ &= \frac{7.386}{0.473 \text{ m}^2/\text{Sec}} \end{aligned}$$

$$\Rightarrow y_{cr} = \left( \frac{(0.473)^2}{9.81} \right)^{1/3} = 0.13$$

As  $y > y_{cr}$   
 $0.58 > 0.13$   
So flow is sub-critical

### Q#3

Given Data:-

Friction Drag ( $F_D = ?$ )

$$\text{width (B)} = 200\text{mm} = 0.2\text{m}$$

$$\text{Length (L)} = 800\text{mm} = 0.8\text{m}$$

$$\text{Specific Gravity (S)} = 0.89$$

$$\text{Undisturbed Velocity (U)} = 5\text{m/sec}$$

$$\text{Kinematic Velocity (v)} = 0.93 \times 10^{-4} \text{ m}^2/\text{Sec}$$

Sol:-

Checking whether flow is laminar or not  
By Reynold Number.

$$R = \frac{DU}{v}$$

For smooth flat plate

$$D = L, \quad v = U$$

$$\text{So } R = \frac{LU}{v}$$

$$= \frac{0.8 \times 5}{0.93 \times 10^{-4}} = 43010$$

$43010 < 500,000 \rightarrow$  Laminar

By Using formula:-

$$F_f = C_f \cdot \rho \cdot \frac{U^2}{2} \cdot BL$$

where

$$C_f = \frac{1.328}{\sqrt{R}} = \frac{1.328}{\sqrt{43010}} = 0.0064$$



$$S = \frac{\rho_{\text{soil}}}{\rho_{\text{water}}} \Rightarrow 0.89 = \frac{\rho_{\text{soil}}}{1000}$$

$$\rho_{\text{soil}} = 0.89 \times 1000$$

$$\rho_{\text{soil}} = 890 \text{ kg/m}^3$$

$$F_f = C_f \cdot \rho \cdot \frac{U^2}{2} \cdot bL$$

$$= 0.0064 \times 890 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = 11.39 \text{ N}$$